Appendix A.7.1 BD02 Standard Overbridges

A.7.1

Galway County Council N6 Galway City Ring Road

Standard Overbridges Preliminary Design Report

GCOB-4.04-020-002

Issue 3 | 23 October 2017

This report takes into account the particular instructions and requirements of our client.

It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.

Job number 233985

Arup Corporate House City East Business Park Ballybrit Galway H91 K5YD Ireland www.arup.com

ARUP

Document Verification

ARUP

Job title		N6 Galway City Ring Road			Job number			
				233985				
Document title		Standard O	verbridges Prelimina	ary Design Report	File reference			
					GCOB-4.04-020-002			
Document 1	Document ref		1-020-002					
Revision	Date	Filename	GCOB-4.04-20-002 (PDR Standard Overbridges)_I1.docx					
Issue 1 28 Oct 2016		Description	Issue 1					
			Prepared by	Checked by	Approved by			
		Name	Yalda Acar	Pat Moore	Eileen McCarthy			
		Signature	Yaldafen	RM.	lileen Mc Carthy.			
Issue 2	30 Jun 2017	Filename	yet.docx					
		Description						
			Prepared by	Checked by	Approved by			
		Name	Yalda Acar	Pat Moore	Eileen McCarthy			
		Signature	Yaldafen	PM.	lileen Mc Cuthy.			
Issue 3	23 Oct 2017	Filename	GCOB-4.04-20-002 (PDR Standard Overbridges)_I3_not yet issued.docx					
		Description	Issue 3					
			Prepared by	Checked by	Approved by			
		Name	Yalda Acar	Pat Moore	Eileen McCarthy			
		Signature	Yaldafen	RM.	Eleen ulberty			
		Filename						
		Description						
			Prepared by	Checked by	Approved by			
		Name						
		Signature						
	I	1	Issue Document Verification with Document					

Page

Contents

1	Introdu	uction	1
	1.1	Design brief given to the authors, including dates	1
	1.2	Background information covering the origins for the need the structure	for 1
	1.3	Previous studies and their recommendations	1
2	Site and	d function	2
	2.1	Site location	2
	2.2	Function of the structure and obstacles crossed	2
	2.3	Choice of location	2
	2.4	Site description and topography	3
	2.5	Vertical and horizontal alignments	5
	2.6	Cross sectional dimensions on the alignments	7
	2.7	Existing underground and overground services	8
	2.8	Geological summary	9
	2.9	Hydrology and Hydraulic summary	10
	2.10	Archaeology summary	10
	2.11	Environmental summary	10
	2.12	Sustainability	10
3	Structu	ire and aesthetics	11
	3.1	Structural Options Considered	11
	3.2	General description of recommended structures	11
	3.3	Aesthetic considerations	23
	3.4	Proposals for the recommended structure of family of structures	23
4	Safety		26
	4.1	Traffic management during construction including land for	•
		temporary diversions	26
	4.2	Safety during construction	26
	4.3	Safety in use	26
	4.4	Lighting	26
5	Cost		27
	5.1	Budget Estimate in current year, including whole life cost	27
6	Design	Assessment Criteria	28
	6.1	Normal Loading	28
	6.2	Abnormal Loading	28

	6.3	Footway live loading	28
	6.4	Provision for exceptional abnormal loads	28
	6.5	Any special loading not covered above	28
	6.6	Heavy or high load route requirements and arrangements being made to preserve route	28
	6.7	Minimum headroom provided	28
	6.8	Authorities consulted and any special conditions required	29
7	Grour	nd Conditions	30
	7.1	Description of the ground conditions and compatibility wit proposed foundations	th 30
8	Drawi	ings and Documents	31
	8.1	List of all documents accompanying the submission	31

Appendices

Appendix A

Drawings

Appendix B

Extract from ground investigation data

1 Introduction

1.1 Design brief given to the authors, including dates

This report has been produced by Arup, who have been appointed by Galway County Council to provide multi-disciplinary engineering consultancy services for the N6 Galway City Transport Project. Galway County Council, Galway City Council, Transport Infrastructure Ireland (formerly known as the National Roads Authority) and the National Transport Authority are collaborating in developing a solution to the existing transportation issues in Galway City and its environs. The solution will include a smart mobility component, public transport component and a road component. The road component of the solution is known as N6 Galway City Ring Road (GCRR).

1.2 Background information covering the origins for the need for the structure

The standard overbridges are required as part of the proposed N6 Galway City Ring Road to preserve regional routes, local routes and access across lands impacted by route of the mainline.

1.3 Previous studies and their recommendations

Previous studies and documents relevant to this Outline Structures Report are listed below.

- Galway County Council. Project Brief. Phase 1, Scheme Concept and Feasibility Studies (REF/14/11222, 2 May 2015).
- Galway County Council. Project Brief. Phase 2, Route Selection (REF/14/11222, 6 November 2015).
- GCOB-4.04-009 Route Selection Report, Issue 1, 16/03/2016
- Galway Transport Strategy, An Integrated Transport Management Programme for Galway City and environs, Technical Report, September 2016

2 Site and function

2.1 Site location

The proposed standard overbridges are located at chainages stated in **Table 1** below, along the mainline of the proposed road development.

Name of Structure	Chainage
S01/01- Forai Maola to Troscaigh Link Overbridge	01+380
S03/01 - Barr Aille Overbridge	03+325
S12/02 – Castlegar Wildlife Overbridge	12+700
S13/01 - School Road Overbridge	13+170
S14/01 - Parkmore Link Road Overbridge	14+375
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	16+420
S16/02 - Coolagh Junction Overbridge (EB merge from R446)	16+860

Table 1Standard overbridge mainline chainages.

2.2 Function of the structure and obstacles crossed

Structure S01/01, S03/01, S13/01 and S14/01 are overbridges required to carry local roads over the proposed road development.

Structure S12/02 is required as a wildlife crossing over the proposed road development.

Structures S16/01 and S16/02 are required at Coolagh junction to provide free flow access between the R446 and the proposed road development.

2.3 Choice of location

S03/01 and S13/01 are located at the position of the crossing between existing local roads with the proposed road development. S14/01 is located at the position of the proposed Parkmore Link Road, and S01/01 is located at the proposed Forai Maola to Troscaigh Link Road.

An ecological assessment was undertaken to determine the location of S12/02 to facilitate the crossing of the proposed road development by bats and is located in an area currently used by bats at Ch. 12+700.

The position of structures \$16/01 and \$16/02 are determined based on the junction design for the interchange at Coolagh.

2.4 Site description and topography

Table 2	Site description and	topography
---------	----------------------	------------

Name of Structure	Site description and Topography		Archaeology at Structure	Ecology at Structure
Structure	N6 Mainline Road over		Structure	
S01/01 Forai Maola to Troscaigh Link Overbridge	At grade or shallow cut of approx 1m.	On embankment approx. 6.5m fill.	None.	Ecological constraint Annex Code 4030 Fosset Code HD1, Local Importance (lower value)
				Fosset Code HH1, National Importance
S03/01 - Barr Aille Overbridge	In cut – approx 4.5m	On embankment approx. 3.7m fill	Site of cultural heritage listed in Appendix 13 of the EIS as CH18 – Vernacular cottage, now extended. Marked on the 1895-1900 mapping. A townland boundary listed in Appendix 13 of the EIS as TB8 – Local road. Boundary between Ballard West and East not extant	None

Name of Structure	Site description and	Topography	Archaeology at Structure	Ecology at Structure
Structure	N6 Mainline Road over		Structure	
S12/02 – Castlegar Wildlife Overbridge	In cut – approx. 7.0m	On embankment approx. 1m fill	None	None
S13/01 – School Road Overbridge	In cutting – approx. 11.3m	At grade or shallow cutting.	None	None

Name of Structure	Site description and Topography		Archaeology at Structure	Ecology at Structure
Structure	N6 Mainline Road over		Structure	
S14/01 - Parkmore Link Road Overbridge	In cutting – approx 11.3m	At grade	None	None
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	In cutting – approx 1m	On embankment approx. 7.5m fill	None	None
S16/02 Coolagh Junction Overbridge (EB merge from R446)	In cut – approx 10.0m	Approx. at grade	A townland boundary listed in Appendix 13 of the EIS as TB32	None

2.5 Vertical and horizontal alignments

Table 3Vertical and horizontal alignments

	N6 Mainline		Overbridges	
Name of Structure	Vertical Alignment	Horizontal Alignment	Vertical Alignment	Horizontal Alignment
S01/01 Forai Maola to Troscaigh Link Overbridge	Gradient 1.92%	Curve R=720	R=1075	R=510
S03/01 - Barr Aille Overbridge	Gradient 1.20%	Curve R=1020	R=1000	R=Straight

	N6 Mainline		Overbridges	
Name of Structure	Vertical Alignment	Horizontal Alignment	Vertical Alignment	Horizontal Alignment
S12/02 – Castlegar Wildlife bridge	Gradient 0.75%	Curve R=2040	R=650	R=Straight
S13/01 - School Road Overbridge	Gradient 0.9%	Curve R=20000	R=1700	R=Straight
S14/01 – Parkmore Link Road Overbridge	Curve R=10000	Curve R=1020	Gradient 1.2%	R=1020
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	Curve R=11000	Curve R=720	R=1700	R=255
S16/02 Coolagh Junction Overbridge (EB merge from R446)	Curve R=70000	Curve R=720	R=5500	R=Straight

2.6 Cross sectional dimensions on the alignments

The proposed cross section of the road under the underbridge structures is given in Table 4 below.

Name of Structure	Carriageway Width[1] (m)	Verge Width (m) [Left]	Verge Width (m) [Right]
S01/01 Forai Maola to Troscaigh Link Overbridge	Maola to Troscaigh Link 12.4		3.0
S03/01 - Bar Allie Overbridge	12.3	3.0	3.0
S12/02 – Castlegar Wildlife Bridge	31.0	3.0	3.0
S13/01 - School Road Overbridge	29.5	3.0	3.0
S14/01 – Parkmore Link Road Overbridge		3.0	3.0
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	Varies 28 min	4.0	3.0
S16/02 Coolagh Junction Overbridge (EB merge from R446)	Varies 28 min	4.0	3.0

Table 4:Cross section dimensions below bridge deck

[1] Carriageway measured from running edge of verge to running edge of verge.

The following cross section details is proposed for the side roads.

Table 5:	Dimensions above bridge deck
----------	------------------------------

Name of Structure	Carriageway Width (m)	Raised Verge Width (m) [Left [1]]	Raised Verge Width (m) [Right [2]]	Parapet width (m) [Left]	Parapet width (m) [Right]	Total (m)
S01/01 Forai Maola to Troscaigh Link Overbridge	6.0	Varies 1.8min	Varies 1.8min	0.5	0.5	11.2
S03/01 - Bar Allie Overbridge	6.0	1.8	1.8	0.5	0.5	10.6

GCOB-4.04-020-002 | Issue 3 | 23 October 2017 | Arup

Name of Structure	Carriageway Width (m)	Raised Verge Width (m) [Left [1]]	Raised Verge Width (m) [Right [2]]	Parapet width (m) [Left]	Parapet width (m) [Right]	Total (m)
S12/02 – Castlegar Wildlife Bridge	6.0	11.5[2]	11.5[2]	0.5	0.5	30.0
S13/01 - School Road Overbridge	6.0	Varies 1.8min	Varies 1.8 min	0.5	0.5	11.5
S14/01 – Parkmore Link Road Overbridge	11.235	Varies 1.85min	Varies 1.85min	0.5	0.5	17.25
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	9.3	2.5	0.6	0.5	0.5	13.4
S16/02 Coolagh Junction Overbridge (EB merge from R446)	9.3	Varies 1.5min	Varies 3.0 min	0.5	0.5	Varies 14.8min

[1] Based on direction of increasing chainage along the side-road alignment.

[2] Wildlife bridge: Space provided for planting and earth bunds.

2.7 Existing underground and overground services

All the utility providers have been consulted with during the preliminary design process. The existing services in the vicinity of the proposed structures are outlined in Table 6 below.

Name of Structure	Existing Services		
S01/01 - Forai Maola to Troscaigh Link Overbridge	MV Overhead Single Phase		
S03/01 - Barr Aille Overbridge	Overhead Eir Services		
S12/02 – Castlegar Wildlife Bridge	None		
S13/01 - School Road Overbridge	Underground Eir Services		
	315 PE Gas,		
	100mm Watermain		
	Public Foul Sewer		
	LV Overhead Single Phase I		
S14/01 Parkmore Link Pood Overbridge	ESB Overhead 38kv		
S14/01 – Parkmore Link Road Overbridge	ESB Overhead 110kv		

Table 6	Existing	Services
	LAISting	Der vices

J:223000/233985-00/4. INTERNALI4-04 REPORTS/4-04-03 INFRASTRUCTURE/20. BRIDGES/2. STANDARD OVERBRIDGES/JSSUE 3/GCOB-4.04-20-002 (PDR STANDARD OVERBRIDGES)_I3.DOCX

Name of Structure	Existing Services
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	ESB Overhead 38kv ESB Overhead 110kv
S16/02 Coolagh Junction Overbridge (EB merge from R446)	ESB Overhead 38kv ESB Overhead 110kv

2.8 Geological summary

Table 7Geotechnical Summary

Name of Structure	Chainage	Average Depth from e.g.l to groundwater level(m)	Average depth from e.g.l to rockhead (m)	Ground Conditions	Preliminary Karst Risk
S01/01 - Forai Maola to Troscaigh Link Overbridge	01+375	0.8	1.0	TOPSOIL / Dark brown PEAT Soft slightly granular SILT Loose to medium dense silty GRAVEL	n/a[1]
S03/01 - Barr Allie Overbridge	03+300	0.5	0.8	Slightly weathered GRANITE Peaty TOPSOIL Slightly weathered GRANITE	n/a[1]
S12/02 – Castlegar Wildlife	12+700	12.7	2.1	Firm to very stiff slightly granular SILT/CLAY Slightly weathered LIMESTONE	High
S13/01 - School Road Overbridge	13+185	7.5	5.1	Firm sandy gravelly CLAY Weathered Rock Slightly weathered LIMESTONE	Medium
S14/01 – Parkmore Link Road Overbridge	14+375	10.0	6.0	Soft sandy SILT Medium dense to dense silty sandy GRAVEL Moderately to slightly weathered LIMESTONE	Medium
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	16+410	2.7	2.9	Firm sandy gravelly CLAY Weathered Rock Slightly weathered LIMESTONE	Low
S16/02 Coolagh Junction Overbridge (EB merge from R446)	16+830	- [2]	1.0	Likely shallow poorly drained Slightly weathered LIMESTONE	Medium

e.g.l. = existing ground level

[1] not applicable as it is an area where the underlying bedrock is granite.

[2] information not available from current ground investigation data

2.9 Hydrology and Hydraulic summary

The groundwater levels are listed in the previous section of this document, see Table 7.

2.10 Archaeology summary

The archaeology summary is provided in Table 2 above.

2.11 Environmental summary

The environmental (ecology) summary is provided in Table 2 above.

2.12 Sustainability

Typically concrete is selected as the primary structural material for the underbridges. Concrete has a high durability performance and requires little maintenance during the design life (120yrs), where the product is appropriated specified and executed. Portland cement replacements such as ground granulated blast-furnace slag (GGBS) will be used where appropriate.

Overbridges S01/01, S03/01, S12/01 and S13/01 are of integral construction. This form of construction minimises the inspection and maintenance requirements compared to non-integral forms of construction. Where the structural arrangement is not appropriate for integral construction, such as S14/01, S16/01 and S16/02, bridges with bearings and expansion joints are adopted.

All structures can be readily demolished at the end of the service life of the bridge, and much of the structural materials (concrete, steel etc) can be recycled and reused.

3 Structure and aesthetics

3.1 Structural Options Considered

At each of the overbridge locations, an assessment of several bridge options have been considered. A brief summary of this assessment is present in Section 3.2 below, along with a description of the recommended structure.

3.2 General description of recommended structures

There are six overbridges identified in the preliminary design of the proposed N6.

3.2.1 Structure S01-01 and 03-01

Structures S01-01 and S03-01 carry side roads over the proposed N6. Structure S03-01 is an overbridge carrying the Aille Road (L5384), over the proposed N6. The crossing is in-line with the existing road, i.e. the plan position of the bridge is at the same location as the existing road, and thus a temporary (local) diversion of the local road will be necessary during the construction works. Structure S01-01 will carry the new Forai Maola to Troscaigh Link over the proposed N6 and will not require any local traffic diversions as it is a new section of side road.

At both these locations, the N6 is a single carriageway cross section. For this structure, the following bridge types have been considered in the option assessment stage:

• Single Span Overbridge

This structure is single span bridge with vertical walls. The deck comprises of pre-cast concrete beams which can be laid to a skew, with an in-situ reinforced concrete deck slab. This is supported with vertical in-situ reinforced concrete walls, see Figure 1.

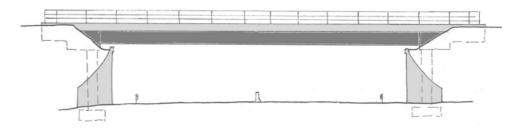


Figure 1: Single span overbridge

• Two Span Overbridge

This structure is a 2 span bridge with a central support and side slopes of 1:2. The deck comprises of pre-cast concrete beams which can be laid to a skew, with an in-situ reinforced concrete deck slab, see Figure 2.

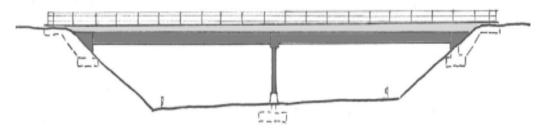


Figure 2: Two span overbridge

• Three Span Overbridge

This structure is 3 span bridge with supports either side of the carriageway and side slopes of 1:2. The deck comprises of pre-cast concrete beams which can be laid to a skew, with an in-situ reinforced concrete deck slab, see Figure 3.

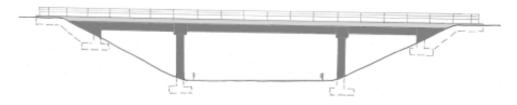


Figure 3: Three span overbridge

The recommended structure is a *three span overbridge*, as this is considered to provide the most appropriate form of construction over a single carriageway mainline and for the rural setting of S01/01 and S03/01.

The single span option does not provide appropriate aesthetics or an open form of construction; the two span overbridge requires a support in the centre of the carriageway which is not appropriate for a single carriageway construction. Alternative forms of a two span bridge, such as asymmetric spans have not been considered at the option development stage.

3.2.2 Structure S12-02

Structure S12-02 is an overbridge structure which provides a wildlife crossing over the proposed N6. The structure is provided with earth bunds to encourage and promote mammal crossing, in particular bats.

Three options have been considered, as described below:

• Twin Arches

A pair of concrete arches provides a system with a large amount of exposed landscaping on the approach to the structure, as indicated in Figure 4 below. It is a structurally efficient solution.

However, due to the span of the arches and the subsequent rise of the arch, the alignment over the mainline needs to be raised, which is undesirable at this location.

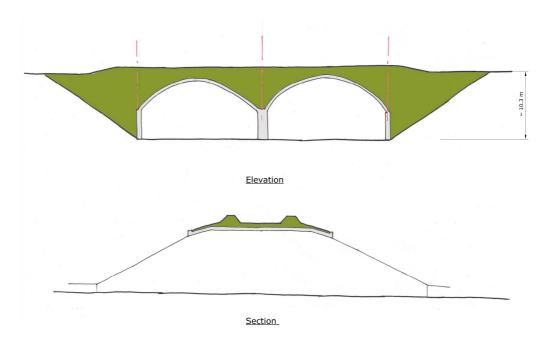


Figure 4: Twin Arches

• Two Span Overbridge with Inverted Deck

In an effort to minimise the need to raise the alignment over the mainline and to minimise the extent of visible structure on the approach, an inverted deck could be adopted, as indicated in Figure 5 below. The inverted deck, uses the space within the earth bund to locate the main longitudinal structural elements, and a lower level deck to support the crossing material.

This is not a conventional form of construction for overbridges and thus would only be justified where the aesthetic need demanded such a solution. At this location, such a demand is not considered to be warranted.

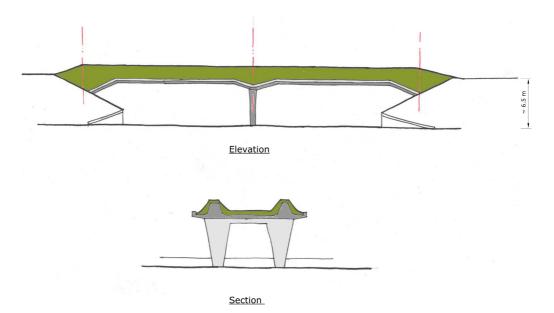


Figure 5: Two Span Overbridge with Inverted Deck Arches

• Two Span Overbridge with Conventional Deck

Where a conventional deck, using precast pre-stressed beams and a concrete deck slab is adopted, the level difference between the crossing and the mainline is a compromise between the two options presented above. This option is presented in Figure 6 below. There is an increased about of structural elements visible on the approaches to the bridge, compared to the other options. However, given the more conventional construction the capital cost will be less than the inverted deck, and from an aesthetic point of view the structure will be comparable to other overbridges on the scheme.

Thus, this option is the recommended form of construction for Structure S12/02.

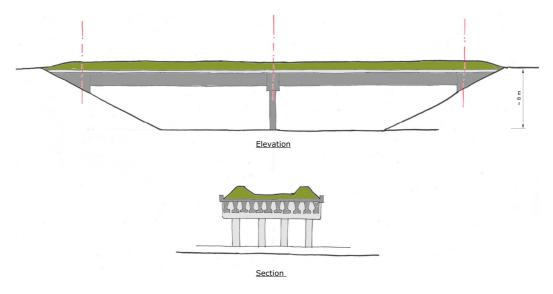


Figure 6: Two Span Overbridge with Conventional Deck

3.2.3 Structure S13-01

Structure S13-01 is an overbridge carrying School Road (L-2134-1), over the proposed N6. Similar to S03/01, the crossing is in-line with the existing road, i.e. the plan position of the bridge is at the same location as the existing road, and thus a temporary (local) diversion of the local road will be necessary during the construction works. At this location, the N6 is a dual carriageway cross section, with an additional weaving lane carriageway in both directions.

Due to the local topography and the proposed alignment, the side road is elevated approximately 11m above the N6. The proposed mainline is in a cutting, with rockhead expected to be approximately 5m above N6 road level. At preliminary design stage, a 1 vertical to 1.5 horizontal slope is adopted in the rock cutting with a 2m horizontal bench at the top of the rockhead, and a 1:2 slope in the overburden material.

The following options have been considered for Structure S13-01.

• Arch bridge

Given the available clearance and the presence of rock at the bottom of the mainline cutting, and arch-type structure is a potential option. Given the relatively short spans, and the compressive arch action, the structure could be constructed of in-situ reinforced concrete. The typical configuration is given in Figure 7 below.

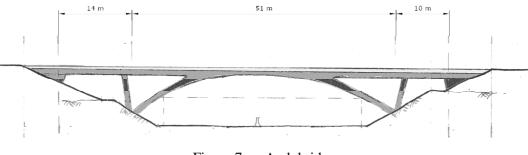
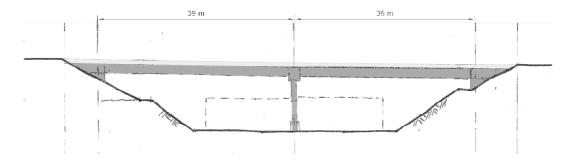
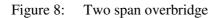


Figure 7: Arch bridge

• Two Span Overbridge

This structure is a 2 span bridge with a central support, as indicated in Figure 8 below. The deck is comprised of pre-cast concrete beams and an in-situ reinforced concrete deck slab. At 39m, the spans are relatively long for this form of construction, but considered to be within the feasible range, in terms of design and construction.





• Three Span Overbridge

This structure is 3 span bridge with supports either side of the carriageway, as shown in Figure 9 below. The deck is comprised of pre-cast concrete beams with an in-situ reinforced concrete deck slab. Due to the skew crossing and presence of weaving lanes, the central span is approximately 40m. Again, this span length is at the upper limit of precast prestressed bridge beams currently in use. Refer to Figure 6.

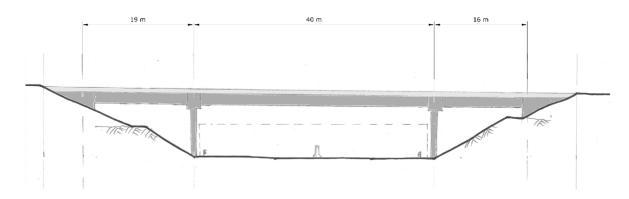


Figure 9: Three span overbridge

The recommended structure is a *two span overbridge*, as this is considered to provide the most appropriate form of construction at this location.

The arch bridge has the potential to provide an attractive and efficient solution, but the geotechnical conditions are more critical and not fully known at this stage of the design.

The three span overbridge is also a feasible alternative. Either of these options could be developed at the next stages of the design as an alternative to the recommended *two span overbridge*.

3.2.4 Structure S14-01

Structure S14-01 is an overbridge carrying the proposed Parkmore Link Road over the proposed N6. At this location, the N6 is a dual carriageway cross section, with additional on and off slip lanes.

With embankment side slopes of 1:1.5, due to the ground conditions present, the overall length of the bridge is approximately 90 to 100m.

Two options have been considered. A two span structure with a steel composite deck (Figure 10); and a 3-span structure with a prestressed precast concrete beam superstructure (Figure 11).



Figure 10: Two span overbridge



Figure 11: Three span overbridge

The recommended structure is a *three span overbridge*, as this is considered to provide the most appropriate form of construction at this location.

Due to the length, bearings and expansions are proposed at the abutments. A fully integral connection between the deck and the pier is envisaged at the intermediate supports. The general arrangement is indicated on drawing GCOB-1700-D-S14-01-001 in Appendix A.

3.2.5 Structure S16-01 and S16-02

Structure S16-01 and S16-02 are overbridge structures at Coolagh Junction, which is the interchange where the proposed N6 Ring Road merges with the existing N6, to the east of Galway City. The overbridges are required to carry merging and diverging lanes over the proposed N6 mainline.

Due to the proximity of the structures to each other, and the similarity in terms of cross section dimensions, length and skew, the options for the two structures are jointly assessed to ensure aesthetic visual consistency. The principle structural

difference between the overbridges is that S16-01 is on a horizontal curvature of approximately 255m, while S16-02 is on a straight highway alignment.

Both overbridges are at a high skew to the N6 mainline. The mainline alignment is on a horizontal curve with a radius of 720m at this location, and consequently widening of the mainline cross section is necessary for forward visibility sightlines.

Several options have been considered as described below. The configurations are presented for S16-01, however the same main principles apply for S16-02, albeit with a straight alignment rather than curved.

• Four Span Overbridge: Precast prestressed beams

This is a 4 span bridge, with a skewed deck using prestressed precast bridge beams, as indicated in Figure 12. The beams are integral at the intermediate supports by means of a downstand crosshead, and bearings and expansion joints are provided at the abutments. With spans up to around 36m, the beam lengths are within the normal range of precast bridge beam construction. Straight bridge beams are adopted resulting in a cross section which varies in width and direction over the length of the bridge.

This form of construction can be readily applied to both S16-01 and S16-02; however due to the high skew crossing the pier crosshead and multiple pier supports make the brides look disproportionally heavy and intrusive in the surroundings.

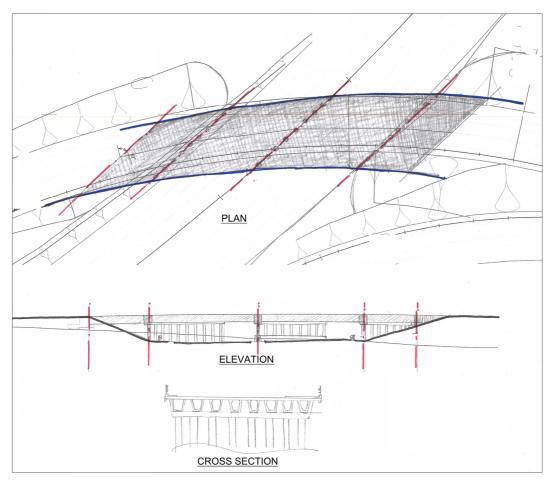


Figure 12 Four span overbridge: Precast prestressed beams

• Four Span Overbridge: Steel composite

This is a 4 span bridge, using steel girders composite with an in-situ deck slab. For S16-01 curved beams can be utilised to minimise the bridge deck width. In addition, the ends of the deck can be made square with the side road alignment which will improve the performance and durability of the expansion joints at the ends of the bridge. For S16-02, a skewed bridge configuration is envisaged.

The steel composite deck could be of ladder deck configuration or using pairs of braced I-girders. For the preliminary design, pairs of I-girders are adopted.

The steel composite deck adopted two columns per pier support location. The reduced number of columns and the diaphragm construction within the depth of the girder provides a more open and slender appearance, compared to the prestressed beam option.

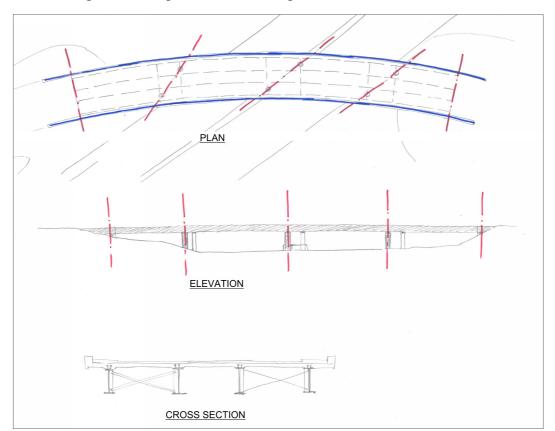


Figure 13 Four span overbridge: Steel composite

• Four Span Overbridge: In-situ post-tensioned deck

This is a 4 span bridge with a post-tensioned, cast in-situ, deck slab, as indicated in Figure 14 below. The use of single column supports at each of the intermediate piers, enables the skew arrangement to be avoided. For S16-01 a curved deck is envisaged; whereas at S16-02 a straight bridge deck can be adopted.

Visually, this configuration is considered to be the most aesthetically pleasing of the options, due to the single column support arrangement and the slender continuous deck. However it is likely to be the most expensive to construct.

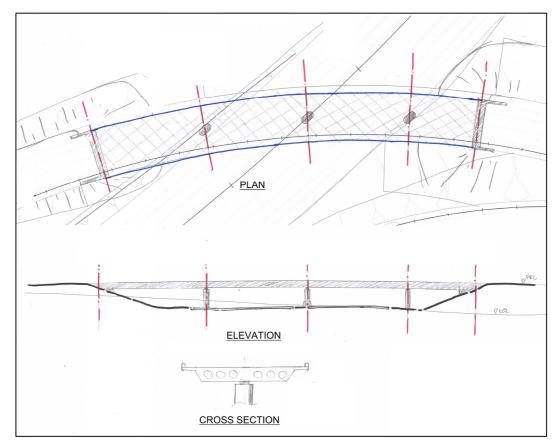


Figure 14 Four span overbridge: In-situ post-tensioned deck

• Two Span Overbridge: Twin Portal Frame

As an alternative to an open overbridges construction, a twin portal frame is considered, as indicated in Figure 15 below. Two portal frames with a spans of approximately 20m and a length of approximately 60m are required to provide a square crossing. Full height walls are required over much of the length and long retaining walls are required at opposing corners of the bridge.

To minimise the tunnel effect of this form of construction, "pergolas" can be utilised on the parts of the bridge deck not carrying the side road directly above, and columns can be used instead of walls where no ground retention is required.

Nevertheless, the twin portal frame results in significant visual impact on the road user due to the closed nature of the abutments and the overall length of the structure along the proposed N6.

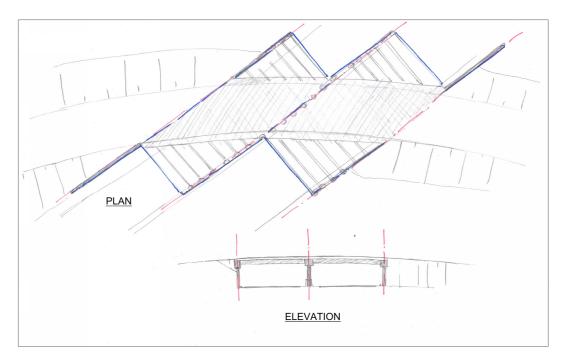


Figure 15 Two span overbridge: Twin Portal Frame

With due consideration of economy and visual impact, the *four span steel composite overbridge* is regarded as the most appropriate option for both S16-01 and S16-02, and is the recommended option further considered in the preliminary design.

3.3 Aesthetic considerations

There is variation in structural form and materials within the overbridge structures on the proposed N6. Nevertheless, a consistent approach will be adopted for each overbridge, in terms of finishes and details, to ensure the aesthetic treatment provides "a family of structures".

3.4 Proposals for the recommended structure of family of structures

3.4.1 Proposed category

The proposed category classification is given in Table 8 below.

3.4.2 Span arrangements

Table 8 Span arrangements

Name of Structure	Span arrangement	Span (m)	Skew angle (deg)	Category
S01/01 - Forai Maola to Troscaigh Link Overbridge	3 Span	10.7 + 17.4 + 10.7	0	2
S03/01 - Bar Allie Overbridge	3 Span	12.5 + 19.1 + 12.5	24	2
S12/02 – Castlegar Green Accommodation Overbridge	2 Span	25.2 + 23.9	0	2
S13/01 - School Road Overbridge	2 Span	39.2 + 35.9	28	2
S14/01 – Parkmore Link Road Bridge	3 Span	25.51 + 32.41 +34.16	29	2
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	4 Span	28.0 + 32.7 + 28.5 + 21.6 [1]	0	3
S16/02 Coolagh Junction Overbridge (EB merge from R446)	4 Span	19[2] + 37.5 + 32.3 + 16.0	58	3

[1] Average span length due to curved bridge deck and varying support line orientation.

[2] Average span length given; deck width varies along the length of the bridge.

3.4.3 Approaches including run-on arrangements

The approach embankments will be constructed using a compacted acceptable material with Clause 6N material behind the abutment walls.

3.4.4 Substructure

The substructure will comprise of in-situ reinforced concrete bankseat abutments and in-situ reinforced concrete columns, crosshead and diaphragms, where applicable, at intermediate supports.

3.4.5 Foundation type

At all the proposed overbridge structures, competent rock is expected at or near the foundations, thus the abutments and central piers will be supported on spread foundations. Where necessary, compacted 6N upfill will be provided between the foundation and the natural ground strata below, to achieve the required bearing capacity.

At some locations there is a medium to high risk of karst features in the ground. Where these occur, appropriate measures will be necessary for the bridge foundations.

3.4.6 Superstructure

• *S01/01, S03/01, S12/01, S13/01 and S14/01*

The superstructure comprises precast prestressed concrete beams with an in-situ reinforced concrete deck slab supported on permanent formwork between the beams and on temporary formwork at the overhanging cantilever edges of the deck slab.

• *S16/01 and S16/02*

The superstructure comprises two pairs of steel I girders, and associated steelwork, with an in-situ reinforced concrete deck slab supported on permanent formwork between the beams and on temporary formwork at the overhanging cantilever edges of the deck slab.

3.4.7 Articulation arrangements, joints and bearings

At structures S01/01, S03/01, and S12/02 the superstructure will be fully integral with the bankseat abutments and central pier.

The continuous steel composite decks at S16/01 ad S16/02 are supported by bearings at the end and intermediate support locations. The concrete superstructure at S13/01 and S14/01 is fully integral at the intermediate support locations and supported on bearings at the abutments. At the abutments, expansion joints are provided.

3.4.8 Parapet

With the exception of S12/02, all the overbridges are typically provided with H2-W4 parapets, 1.25m high, with mesh infill. The approach and departure safety barrier and transitions will be H2 containment.

At structure S12/02 pedestrian parapet will be provided at headwalls and wingwalls in accordance with TII DN-STR-03011 (NRA BD 52).

Due to the direct proximity of school facilities adjacent to structure S13/01, the risk of children climbing the bridge parapet, with the possibility of falling has been identified. Measures will be implemented to mitigate this risk, typically by adopting a higher parapet (e.g. 1.8m) and using solid infill cladding. The performance requirements of the anti-climbing measures will be defined in the next stages of the design and procurement process.

3.4.9 Inspection and maintenance

Structures S01/01, S03/01, and S12/02 are of fully integral concrete construction throughout, and have no movement joints or bearings and hence minimal maintenance requirements are expected.

At structures \$13/01, \$14/01, \$16/01 and \$16/02 the expansion joints and bearings will have inspection and maintain requirements associated with these elements. The appropriate access and inspection facilities will be provided at these structures.

Structures S16/01 and S16/02 utilise structural steel which will require maintenance during the design lifetime (where weathering steel is not used).

For all overbridge structures, access to the bridge soffit will be from the local road below and will require local diversions and a mobile elevated work platform for access purposes.

Waterproofing systems, joints, parapets etc shall be designed for Working Life Category 2 (replaceable structural parts, up to 50 years design working life).

All other elements of the structure shall be designed for Working Life Category 5 (≥120 years design working life).

4 Safety

4.1 Traffic management during construction including land for temporary diversions

Detailed traffic management proposals will be developed at detail design stage by the appointed Contractor in consultation with their Designers and the consent for the diversions and or road closures will be sought from the appropriate local authority.

4.2 Safety during construction

The Designer will take account of the General Principles of Prevention, as specified in the Schedule 3 of the Safety, Health and Welfare at Work Act 2005, liaise with the Project Supervisor appointed by the Client for the Design Process and the Project Supervisor appointed for the Construction Stage and carry out all other duties as required by Clause 15 of the Safety, Health and Welfare at Work (Construction) Regulations 2013 (S.I. No. 291 of 2013).

The Project Supervisor for the Design Process will comply with all the requirements outlined in Clauses 11, 12, 13 & 14 of the Safety, Health and Welfare at Work (Construction) Regulations 2013 (S.I. No. 291 of 2013).

4.3 Safety in use

Parapets and safety barriers will be provided across the length of the structure and on the approach to, and departure from, the structure.

Where applicable, pedestrian protection will be provided at the top of headwalls and wingwalls in accordance with TII DN-STR-03011 (NRA BD 52).

At structure S13/01, a risk assessment has been undertaken and there is a need for additional pedestrian containment measures, at this location See also Section 3.4.8.

4.4 Lighting

No lighting is proposed at S01/01, S03/01, S12/02 and S13/01. Lighting is proposed at the overbridges (S16/01 and S16/02) in the vicinity of Coolagh grade separated junction. Lighting is also proposed at S14/01.

5 Cost

5.1 Budget Estimate in current year, including whole life cost

Table 9 Budget estimate

Name of structure	Structure Cost Excl. VAT (Million Euro)
S01/01- Forai Maola to Troscaigh Link Overbridge	0.6
S03/01 - Barr S01/01 - Forai Maola to Troscaigh Link Overbridge Aille Overbridge	0.7
S12/02 – Castlegar Wildlife Bridge	2.1
S13/01 - School Road Overbridge	1.3
S14/01 – Parkmore Link Road Overbridge	1.9
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	3.3
S16/02 – Coolagh Junction Overbridge (EB merge from R446)	3.2

The cost estimate values given in Table 9 are based on a cost rate per square metre of structure area given in Table 10 below. A range of -10% to +15% is considered to be applicable to the budget cost for this stage of the design.

Table 10 Basis of cost estimate

Structure Type	Rate (Euro/m2)	
Prestressed beam deck overbridge	1450	
Steel composite deck overbridge	2000	

6 Design Assessment Criteria

6.1 Normal Loading

Permanent Actions in accordance with IS EN 1991-1-1:2002 and the associated National Annex.

The structure will be designed for Load Models LM1 and LM2 in accordance with IS EN 1991-2:2003 and the associated National Annex.

All support columns to be designed for full impact loading in accordance with Table NA1 loading of IS EN 1991-1-7

6.2 Abnormal Loading

Load Model 3 up to and including SV196 (where applicable) will be considered in accordance with IS EN 1991-2:2003 and the associated National Annex.

6.3 Footway live loading

Where applicable, a footway loading shall be in accordance with Clause 5.3.2.1 of IS EN 1991-2:2003. A nominal $q_{fk} = 5kN/m2$ will be adopted.

6.4 **Provision for exceptional abnormal loads**

Not applicable.

6.5 Any special loading not covered above

No exceptional abnormal loads are proposed.

6.6 Heavy or high load route requirements and arrangements being made to preserve route

Not applicable.

6.7 Minimum headroom provided

The minimum headroom clearance for underbridge structures will be 5.3m in accordance with TII DN-GEO-03036 (NRA TD 27).

6.8 Authorities consulted and any special conditions required

Consultation with relevant authorities is on-going. The following groups have been contacted as part of the scheme:

Transport Infrastructure Ireland (TII)

Galway County Council (GcoC)

Galway City Council (GciC)

Land and home owners

7 Ground Conditions

7.1 Description of the ground conditions and compatibility with proposed foundations

Name of Structure	Foundation Type	Soil / Rock at Formation Level Pier Abutment		Safe Bearing Pressure (kN/m²)Formation depth above (+)/ below (-) egl (m)PierAbutment		Typical depth to rockhead from egl	
						Abutment	(m)
S01/01 - Forai Maola to Troscaigh Link Overbridge	Pad	Struct ural Upfill / Rock	Structural Upfill / Rock	360	-1.7	+3.1	2.1 (Varies)
S03/01 – Barr Aille Overbridge	Pad	Rock	Rock	370	-6.9	-1.2	0.8
S12/02 – Castlegar Wildlife Bridge	Pad	Rock	Rock	385	-7.0	-7.0	2.0
S13/01 – School Road Overbridge	Pad	Rock	Rock	375	-13.9	-5.3	5.0
S14/01 – Parkmore Link Road Overbridge	Pad	Rock	Rock	335	-13.4	Varies - 5.3min	8.0
S16/01 – Coolagh Junction Overbridge (EB diverge to R446)	Pad	Rock	Soil	165[1]	Varies -2.0 min	Varies +4.3 max	2.9
S16/02 – Coolagh Junction Overbridge (EB merge from R446)	Pad	Rock	Soil	165[1]	Varies -2.9 min	Varies +2.7 max	1.0

Table 11 Ground conditions

egl = existing ground level

[1] Minimum safe bearing capacity (at south abutment)

8 Drawings and Documents

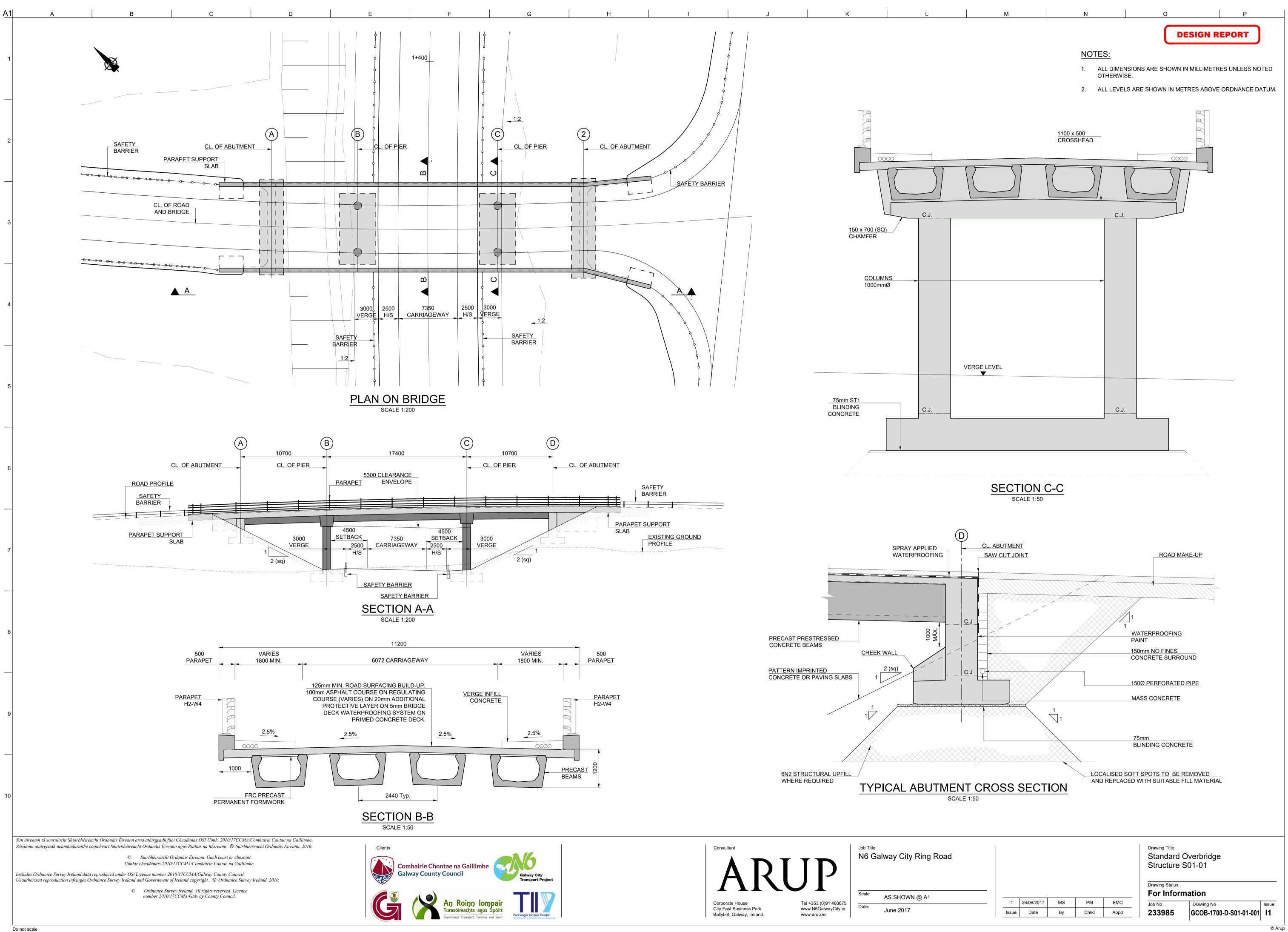
8.1 List of all documents accompanying the submission

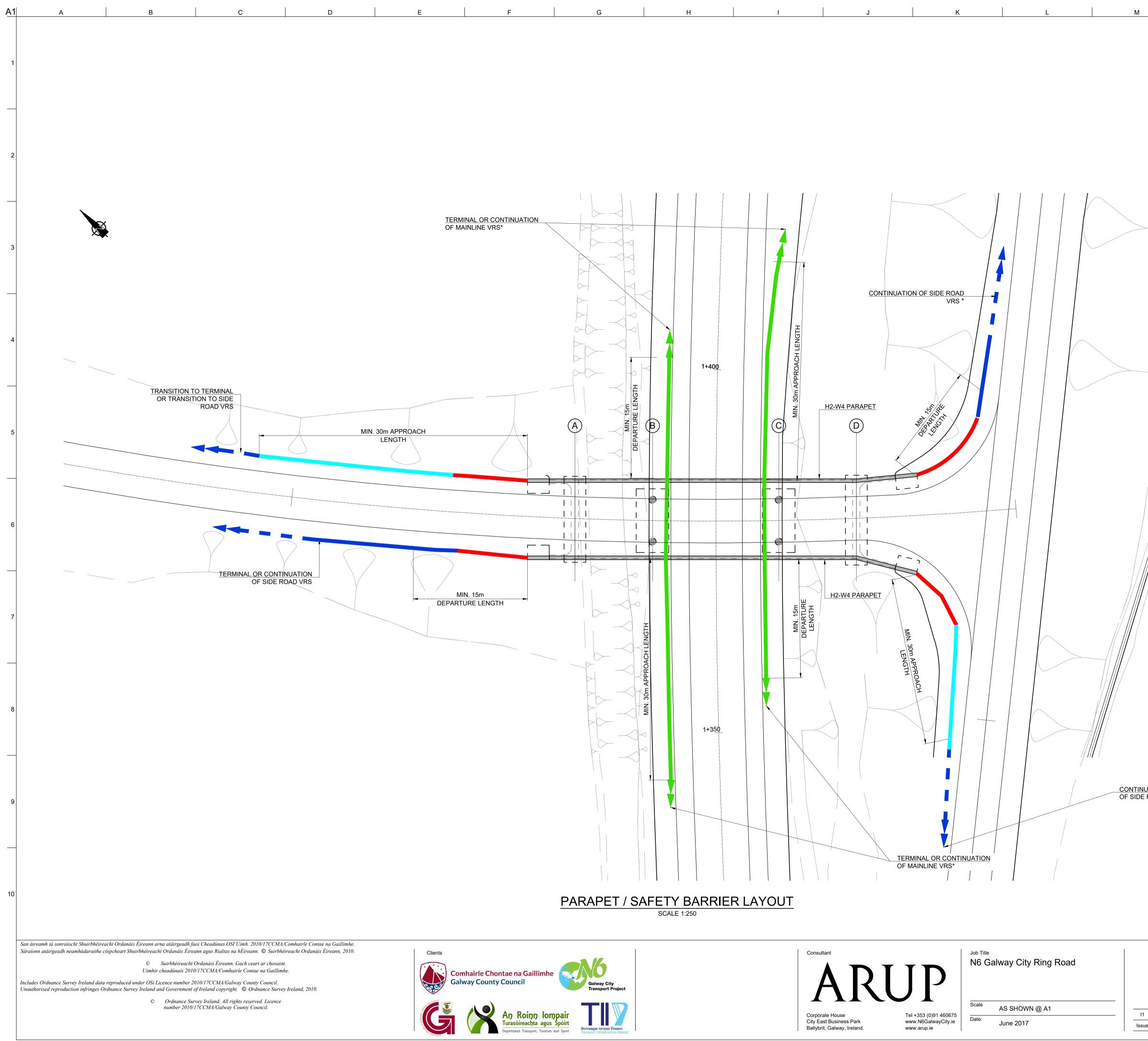
Name of Structure	Drawing Number
Standard Overbridge – Structure S01/01	GCOB-1700-D-S01-01-001
	GCOB-1700-D-S01-01-002
Standard Overbridge – Structure S03/01	GCOB-1700-D-S03-01-001
	GCOB-1700-D-S03-01-002
Standard Overbridge – Structure S12/02	GCOB-1700-D-S12-02-001
Standard Overbridge – Structure S13/01	GCOB-1700-D-S13-01-001
	GCOB-1700-D-S13-01-002
Standard Overbridge – Structure S14/01	GCOB-1700-D-S14-01-001
	GCOB-1700-D-S14-01-001
Standard Overbridge – Structure S16/01	GCOB-1700-D-S16-01-001
	GCOB-1700-D-S16-01-002
Standard Overbridge – Structure S16/02	GCOB-1700-D-S16-02-001
	GCOB-1700-D-S16-02-002

Table 12 Drawings

Appendix A

Drawings



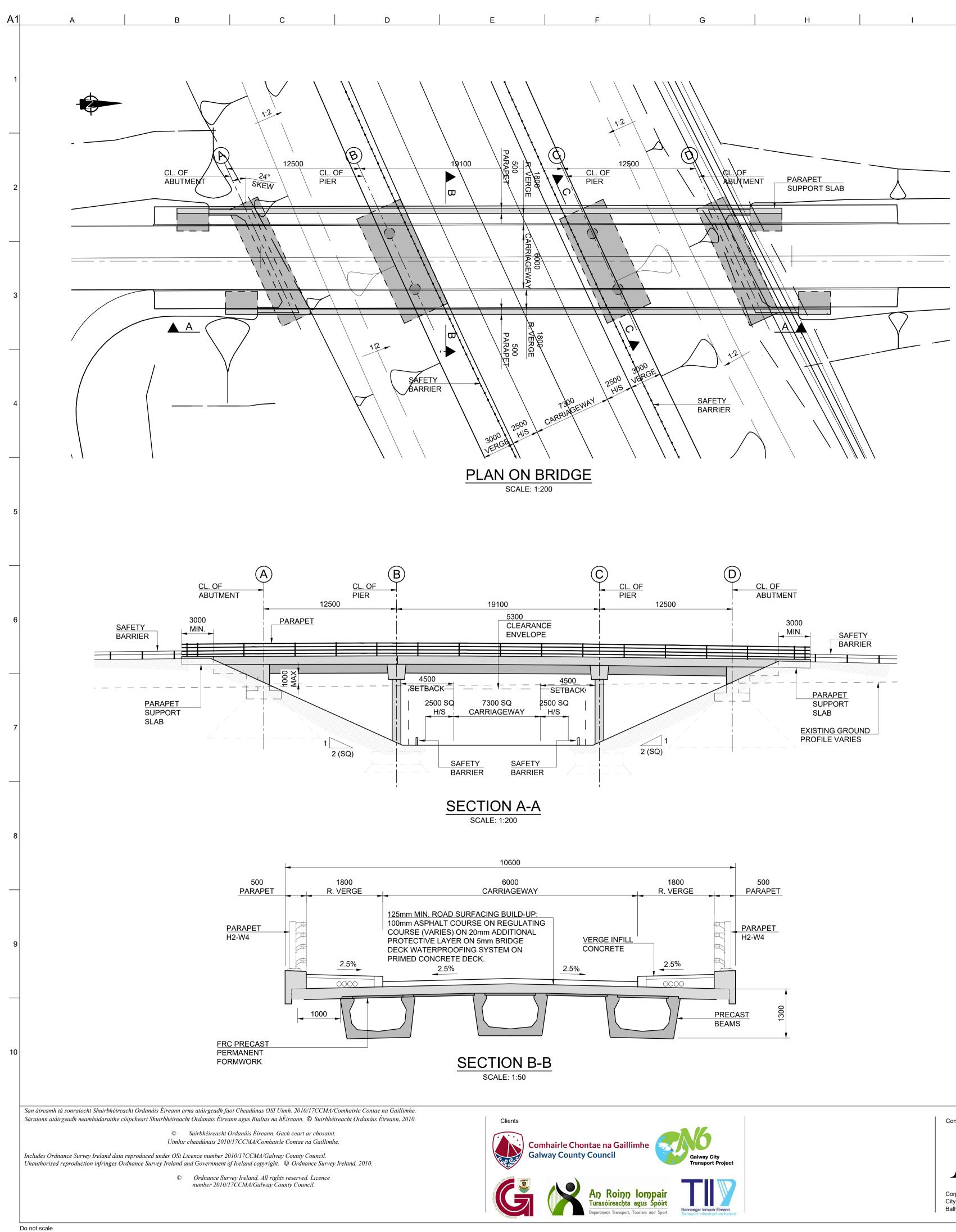


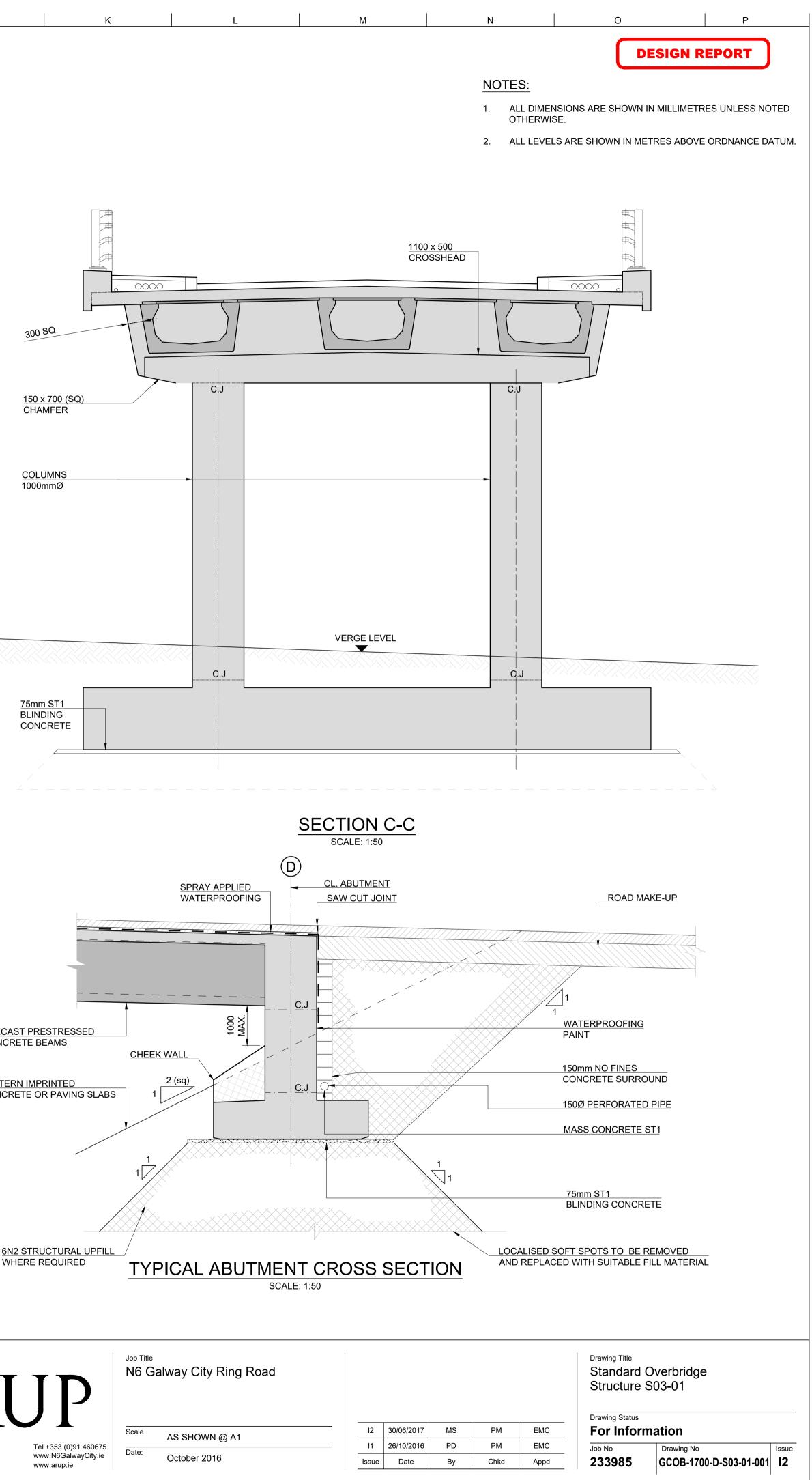
Do not scale

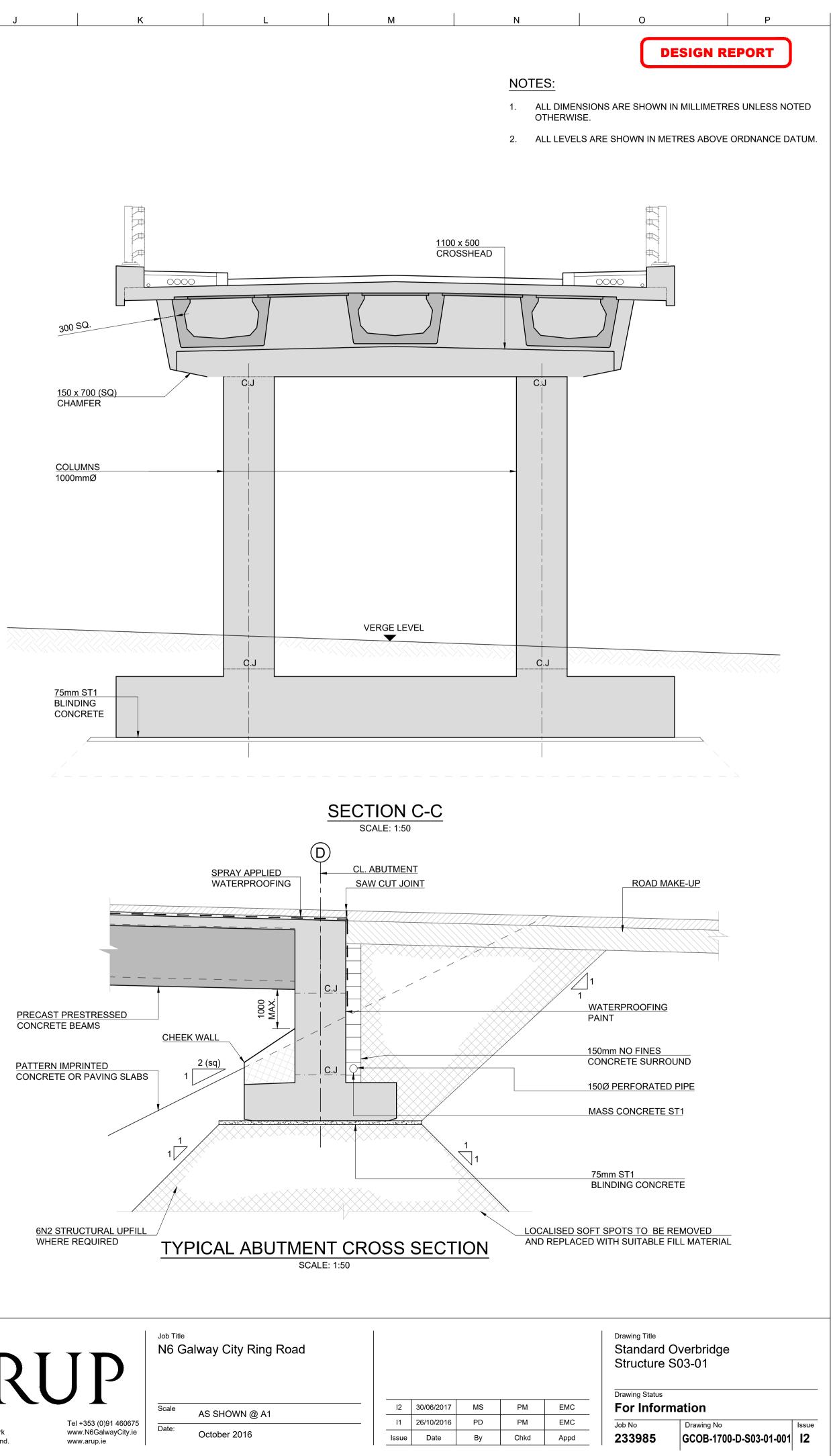
Ν		0	P
		DE	SIGN REPORT
NOTE	ES:		
			CT TO CONFIRMATION DURING ES 400 CERTIFICATION
	LEGE	END:	
		APPROVED PARAPET TC	TRANSITION FROM H2 VRS
		SIDE ROAD \	/RS
		H2 VRS TO D	N-REQ-03034
		MAINLINE BA	ARRIER TO SERIES 400
		H2 - W4 PAR/ DN-STR-0301	

CONTINUATION OF SIDE ROAD VRS*

					Structure	Safety Barrier Layout	
11	30/06/2017	MS	РМ	EMC	Job No	Drawing No	Issue

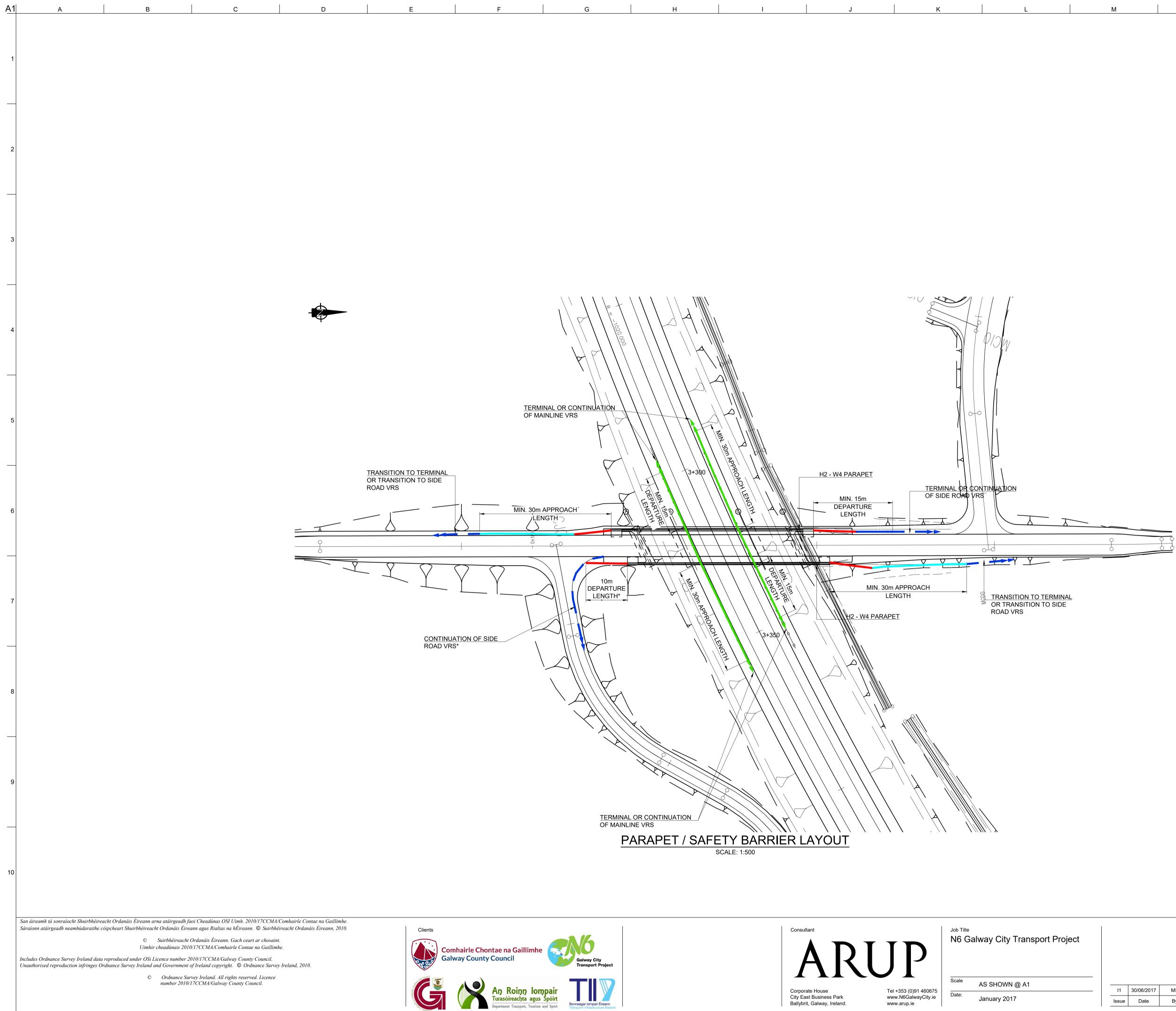






Consultant Corporate House City East Business Park Ballybrit, Galway, Ireland.

© Arup

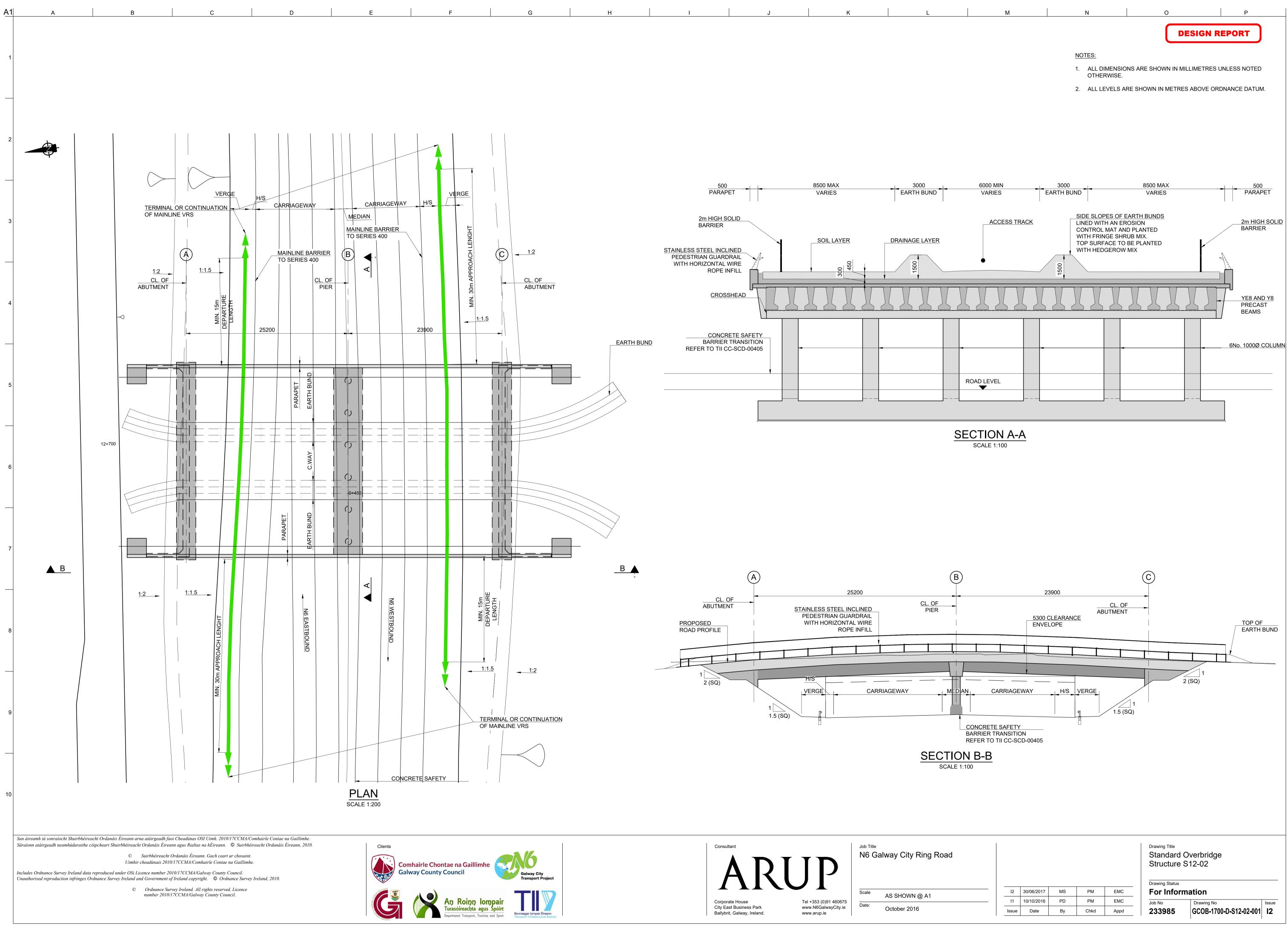


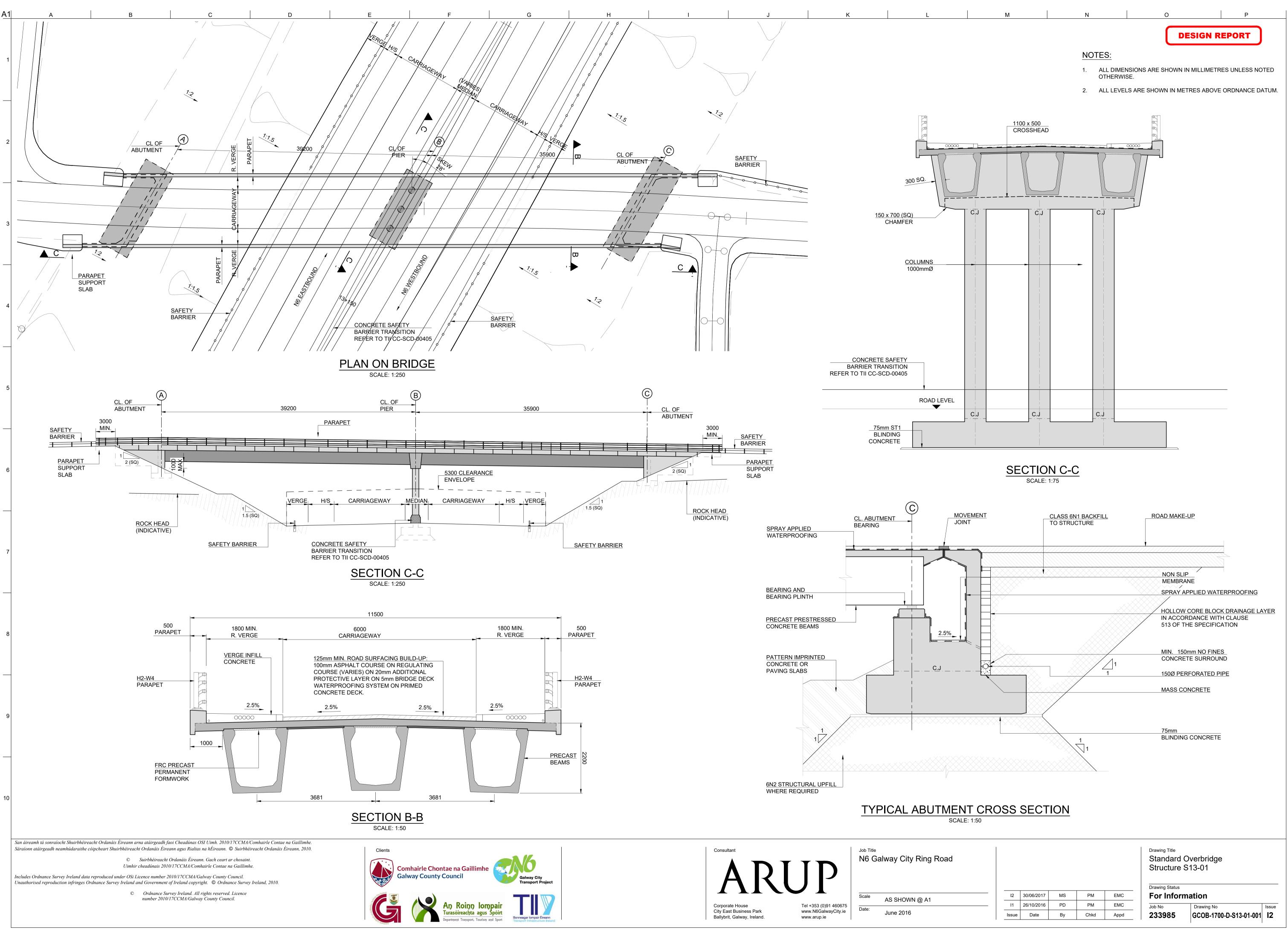
Do not scale

N	0	Р
	DESIGN R	EPORT
NOTES:		
	HOWN ARE SUBJECT TO CONFIRM DESIGN AND SERIES 400 CERTIFIC	
LEGEND:		
	APPROVED TRANSITION FROM PARAPET TO VRS	I H2
	SIDE ROAD VRS	
	H2 VRS TO DN-REQ-03034	
	 MAINLINE BARRIER TO DN-REQ-03034 H2-W4 PARAPET TO DN-STR-03 	011
CERTIFI	AD & MAINLINE VRS DETAILS WILL ED IN 400 SERIES. T TO APPROVAL OF REALXATION.(URE	

М

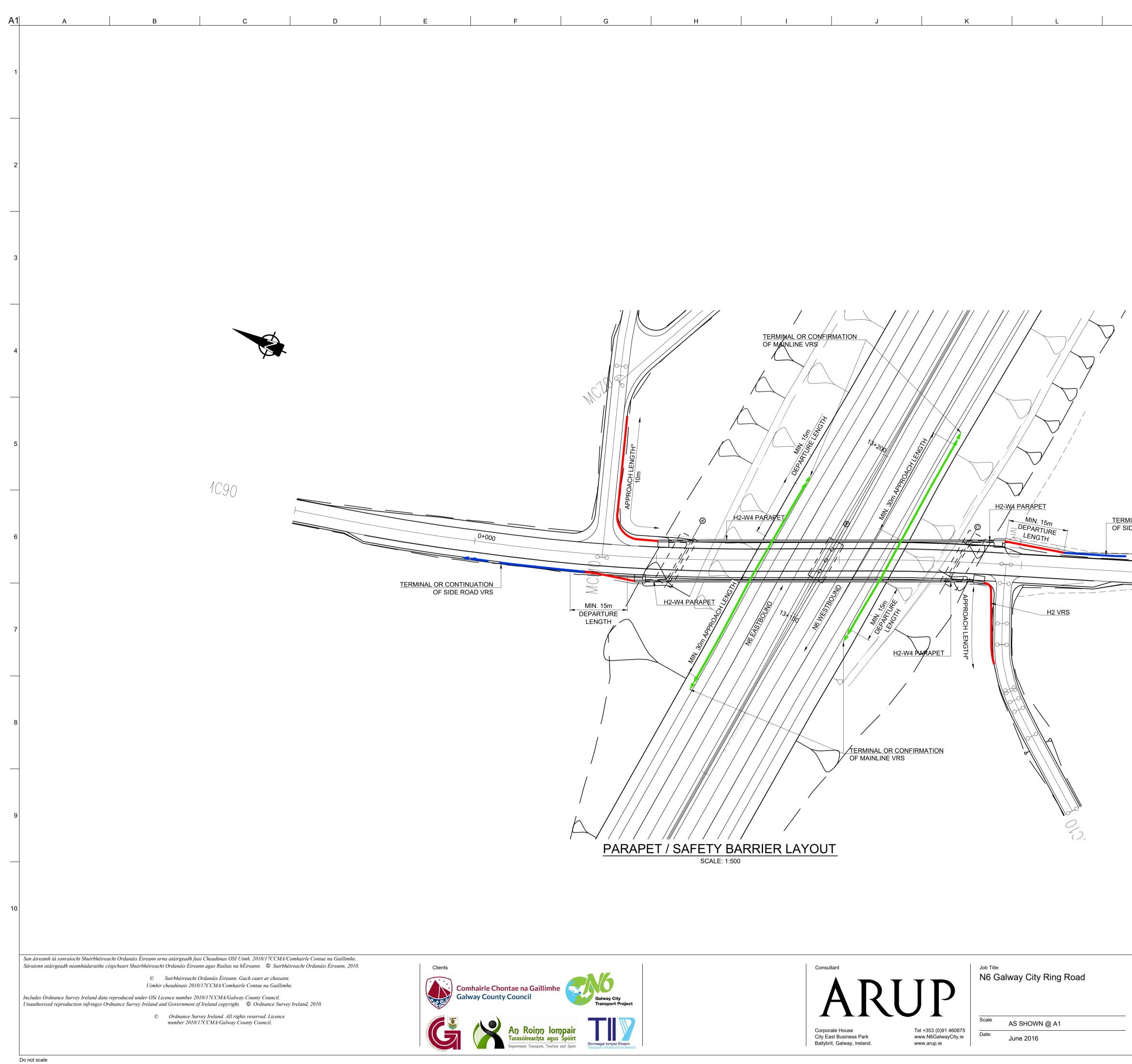
					Drawing Title Standard Structure	Overbridge S01-01	
					Drawing Status For Infori	mation	
11	30/06/2017	MS	PM	EMC	Job No	Drawing No	Issue
Issue	Date	By	Chkd	Appd	233985	GCOB-1700-D-S03-01-002	11





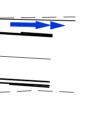
Do not scale

© Arup



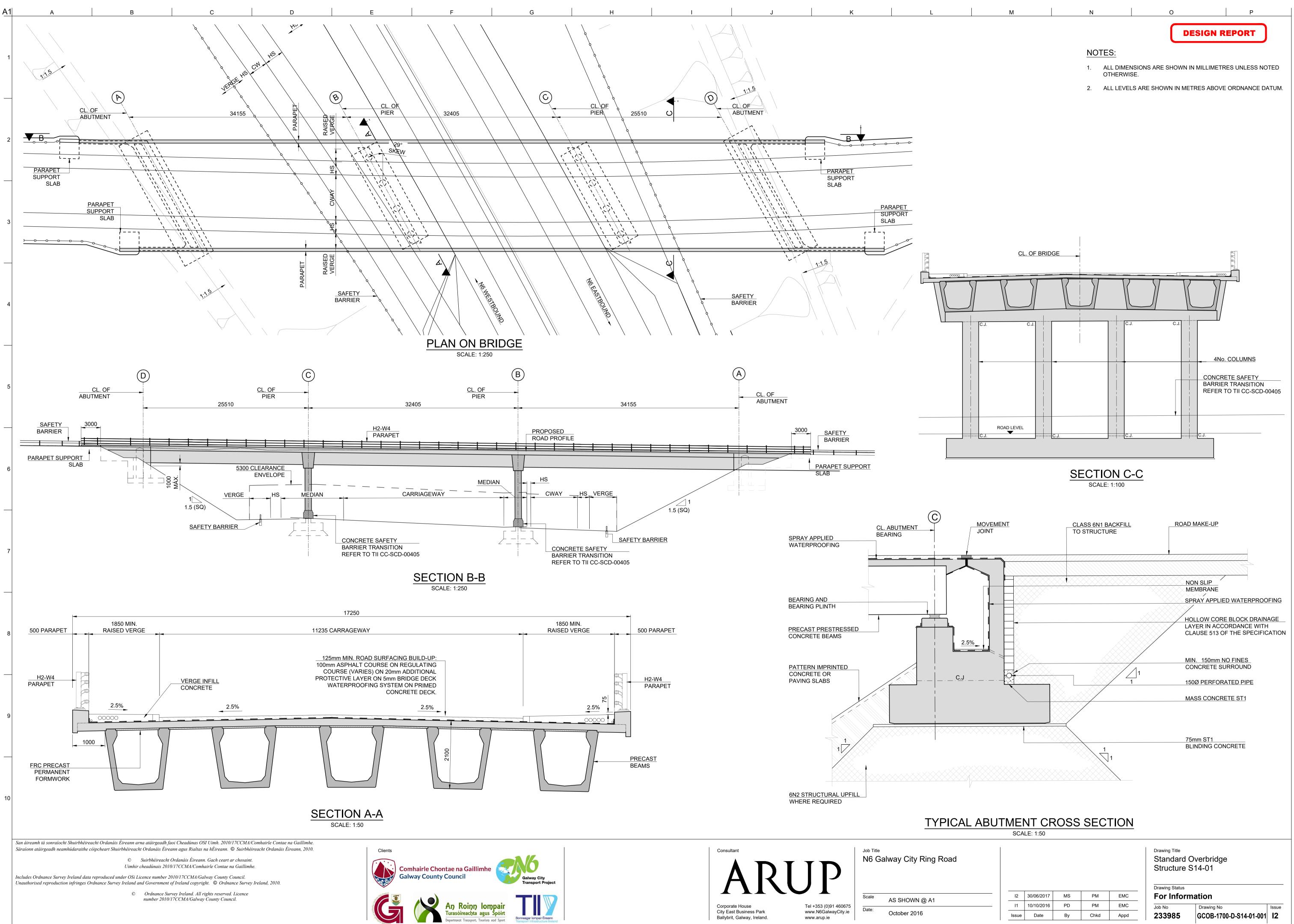
М	N		0		Р
				ESIGN REI	PORT
	NOT	ES:			
	1.	ALL DETAILS SHO THE DETAILED DE			
		LEGEND:			
			APPROVEI PARAPET	D TRANSITION F	ROM H2
			SIDE ROAL	VRS	
			H2 VRS TC	DN-REQ-03034	
			MAINLINE	BARRIER TO SE	RIES 400
			H2-W4 PAF	APET TO DN-S	TR-03011
		* SUBJEC APPRO		ION OR DEPAR	TURE

TERMINAL OR CONTINUATION OF SIDE ROAD VRS



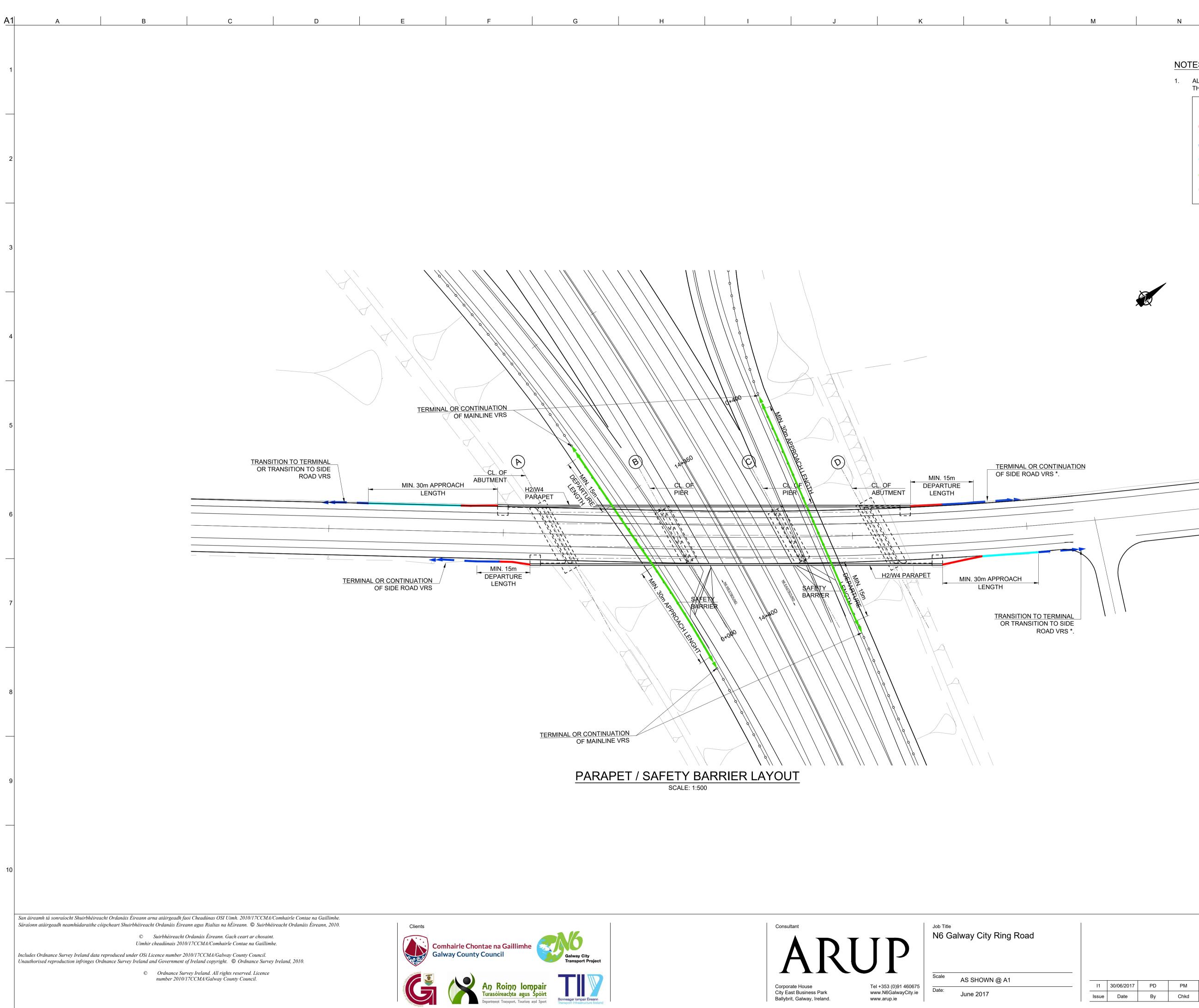
06)W

					Drawing Title Standard Overbridge Structure S13-01 Parapet / Safety Barrier Layout	
					Drawing Status For Information	
11	30/06/2017	PD	РМ	EMC	0	Issue



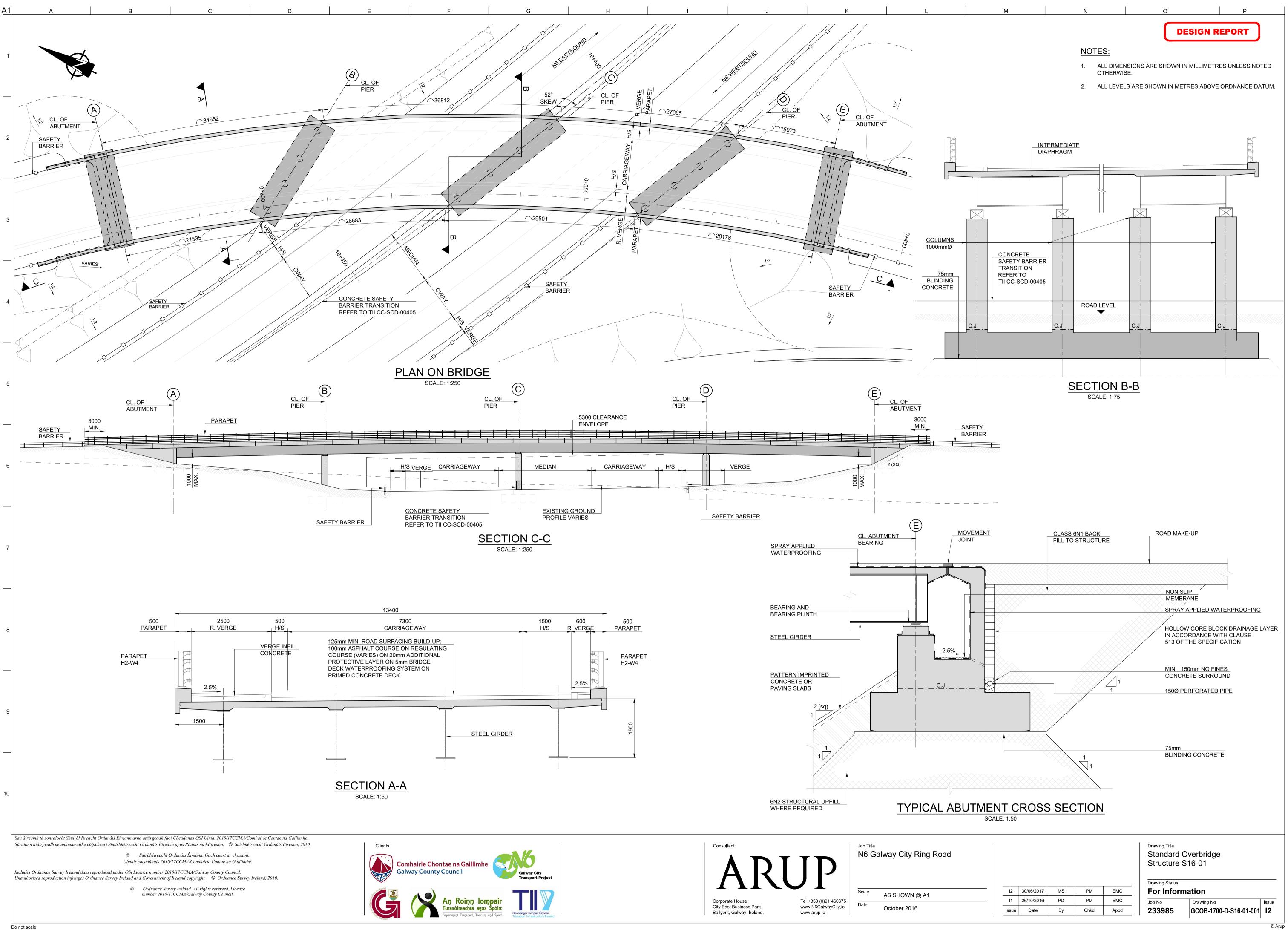
Do not scale

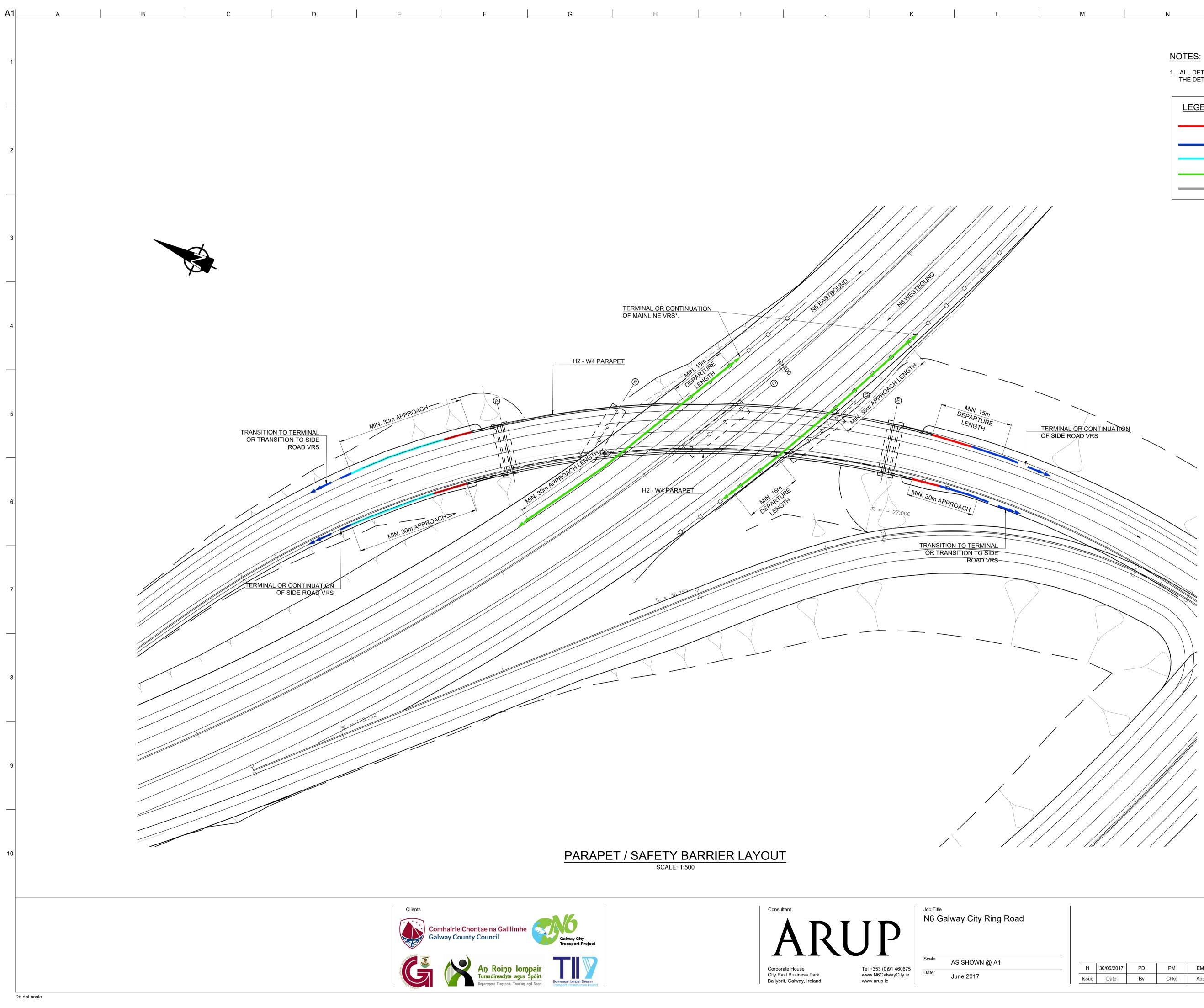
© Arup



И	N		0		Р	
			DE	SIGN R	EPORT	
	NOT	ES:				
	1.		HOWN ARE SUBJE DESIGN AND SERI			RING
		LEGEND	<u>:</u>			
			APPROVED PARAPET TO		N FROM H2	
			SIDE ROAD	VRS		
			H2 VRS TO I	DN-REQ-030)34	
				ARRIER TO	SERIES 400	
			H2-W4 PARA	APET TO DN	I-STR-03011	

					Drawing Title Standard Structure	Overbridge S14-01	
					Drawing Status		
					For Inform	nation	
11	30/06/2017	PD	PM	EMC	For Inform	Drawing No	Issue





Μ	Ν

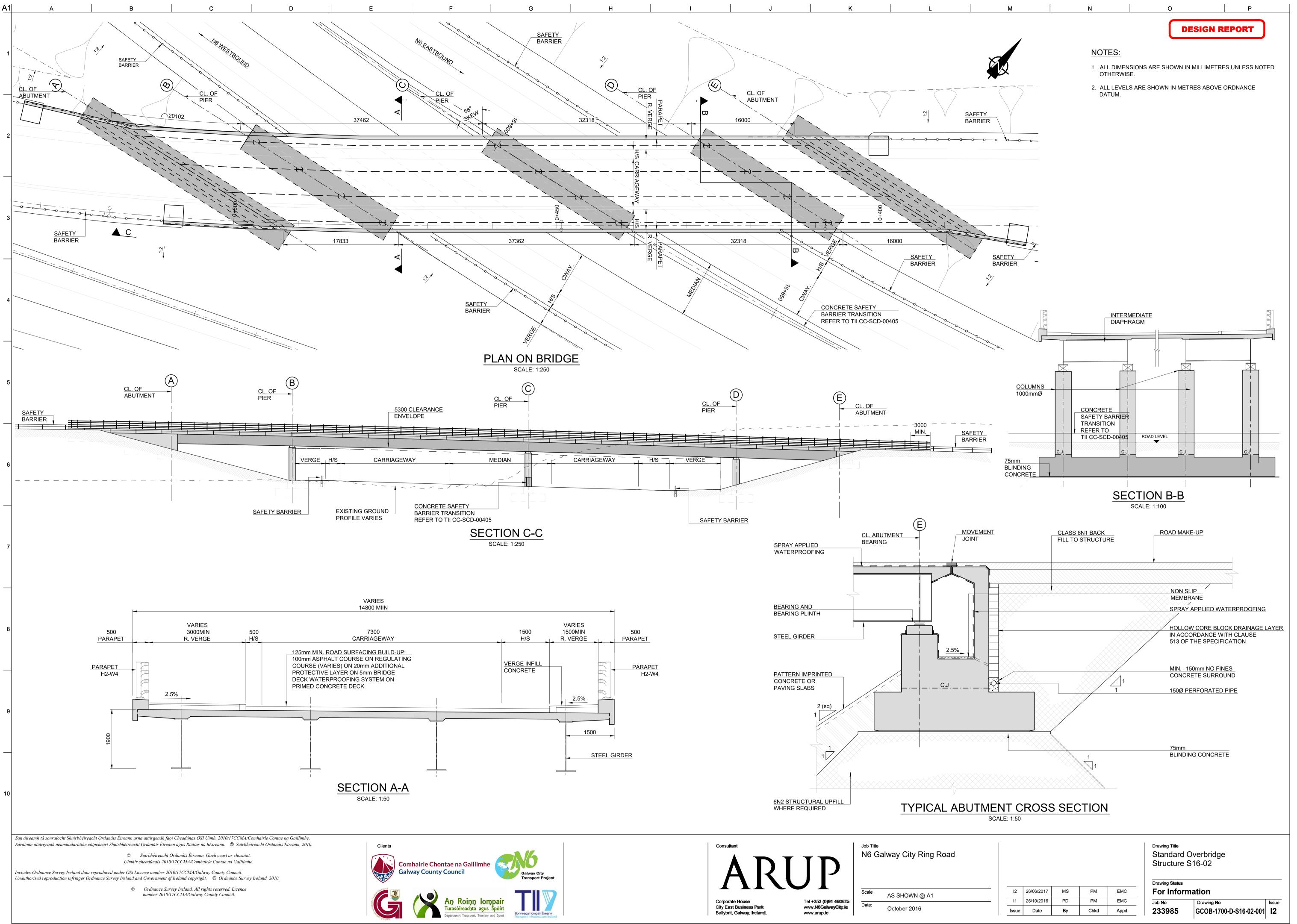


DESIGN REPORT

ALL DETAILS SHOWN ARE SUBJECT TO CONFIRMATION DURING THE DETAILED DESIGN AND SERIES 400 CERTIFICATION.

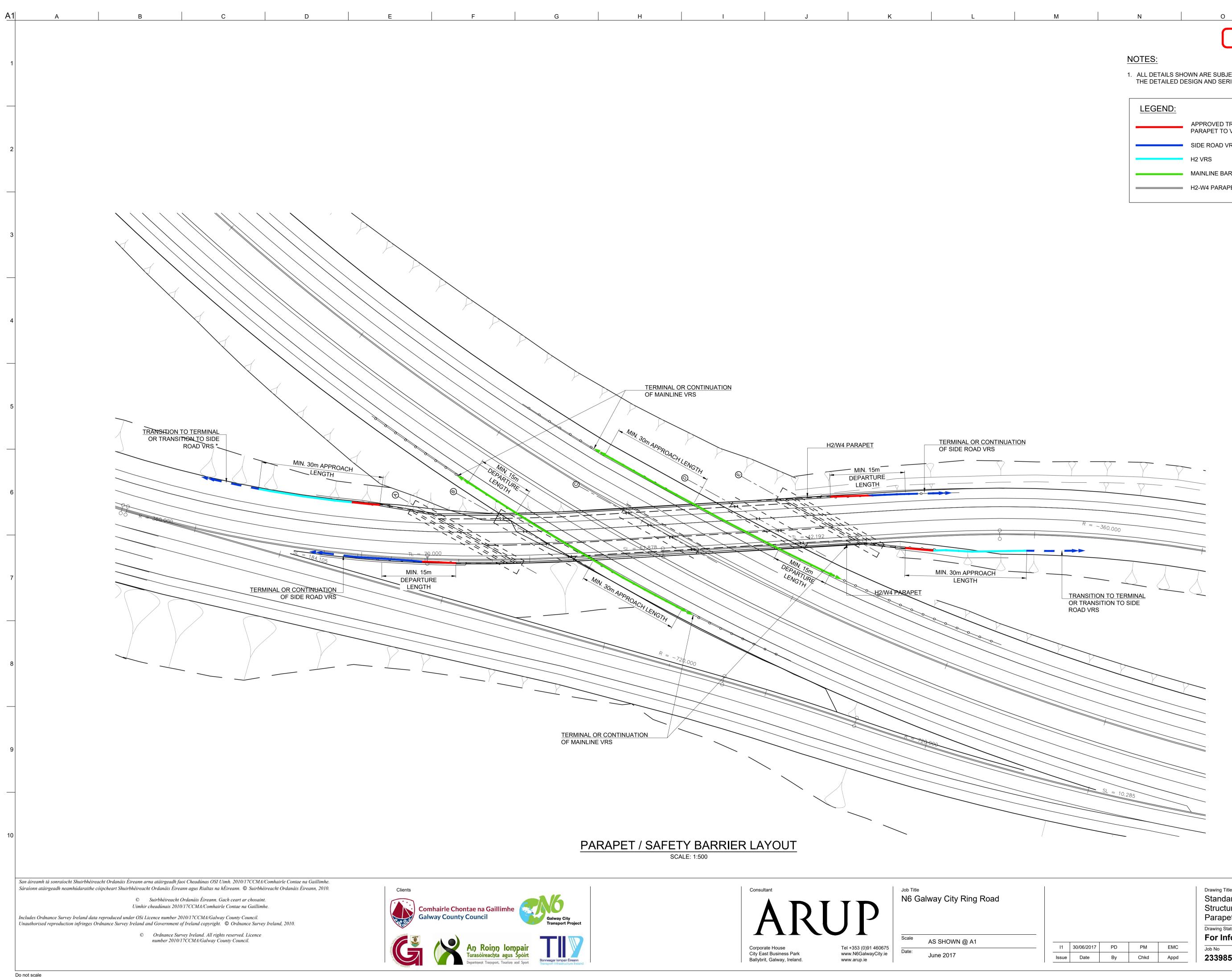
LEGEND:	
	APPROVED TRANSITION FROM H2 PARAPET TO VRS
	SIDE ROAD VRS *
	H2 VRS
	MAINLINE BARRIER TO SERIES 400
	H2-W4 PARAPET

					Structure	Overbridge S16-01 Safety Barrier Layout	
					Drawing Status For Infori	nation	
11	30/06/2017	PD	PM	EMC	0	Drawing No	Issue



Do not scale

© Arup



	DESIGN REPORT
NOTES:	
	OWN ARE SUBJECT TO CONFIRMATION DURING ESIGN AND SERIES 400 CERTIFICATION.
LEGEND:	
	APPROVED TRANSITION FROM H2 PARAPET TO VRS
	SIDE ROAD VRS
	H2 VRS
	MAINLINE BARRIER TO SERIES 400
——	H2-W4 PARAPET

Р

11	30/06/2017	PD	РМ	EMC	Drawing Status For Infor		1
					Structure	Overbridge S16-02 Safety Barrier Layout	
					Drawing Title	Overbridge	

Appendix B

Extract from ground investigation data

B1 Extract from ground investigation data

Reference	Name	Chainage	Ground Investigation
S01/01	Forai Maola to Troscaigh Link Overbridge	01+380	TP3-06, TP3-07, TP3-34
S03/01	Barr Aille Overbridge	03+300	BH3/04R
S12/02	Castlegar Wildlife Overbridge	12+700	BH3/46R, TP3/27
S13/01	School Road Overbridge	13+185	BH3/33R, GP3/12, GP3/18
S14/01	Parkmore Link Road Overbridge	14+375	TP3/22, S14 (P2C1), BH3/47R
S16/01	Coolagh Junction Overbridge (EB diverge to R446)	16+410	BH3/42R, TP3/30
S16/02	Coolagh Junction Overbridge (EB merge from R446)	16+830	



REPORT NUMBER

/	\checkmark																	
	NTRAC		6 Gal	lway City	y Transpor	t Project - F								BORE		NO.	BH3/04 Sheet 1 of 1	
		NATES LEVEL (m A0	523,64 724,28 OD)	5.55 E 6.79 N 36.82	E		e Dle Diam Dle Dept		(mn	n) 2	Dando 30 200).10	000		COM		ED 26/01/2016	
	IENT GINEEF		alway RUP	y County	/ Council			IMER RE						BORE		D BY	WC JL	
		. , .						101110 ()	/0,					nples				
Depth (m)				Desc	cription			Legend	·	Elevation	Depth (m)	Ref. Number	Sample Type	· ·	(111)	Recovery	Field Test Results	Standpipe Details
0	TOP							<u>Ale Ale</u>	36.7	2	0.10							
				sible Roo t 0.10 m														
		Deren	ole u															
1																		
2																		
3																		
4																		
5																		
-																		
6																		
7																		
8																		
9																		
HA	ARD ST	RATA B	ORIN	NG/CHIS	ELLING											WA		
		To (m)	Ti	me c	omments			Wate		Casii		Sealed	Ris		Time		omments	
).1	0.1		<u>h)</u> 1				Strik		Dep	in	At	Tc		(<u>min)</u>			
																	No water strike	
											ole	Casing		onth to			UNDWATER PROC	GRESS
		TION DE			D 75	-		Dat	te		epth	Depth	Ne Ne	pth to /ater	Con	nment	ts	
	Date	Tip De	pth	RZ Top	RZ Base	Туре	9	\neg										
RE	MARK	Boreh	ole b	ackfilled w-on cor	l upon com	npletion. Bo	orehole s	cheduled	for		Samp	Disturbed (tub)	d			UT - Und	disturbed 100mm Diameter	
		rotary	.0101		y.						LB - Larde	Disturbed e Bulk Disturbe ironmental San	d	+ Vial + Tuls		Sample P - Undi	sturbed Piston Sample ter Sample	
											LIV - ENV	nonmental Sar	inhie (Jal	• vidi = IUD	,	vv - vval	o, oampio	



REPORT NUMBER

	ଟ୍ୟ	<u>ل</u> ے														
co	NTR	АСТ	N	6 Ga	Ilway City Transp	ort Pr	oject -	Phase 3			DRILL	HOLE	NO	BH	3/04R	
co	-ORD	DINA	TES		523,645.55 E						SHEE				et 1 of	
				(mOl	724,286.79 N	2		RIG TYPE FLUSH		Knebel Air/Mist	DATE DATE				2/2016 2/2016	
	ent Ginei	ER		ialwa RUP	y County Counci	I		INCLINATION (deg) CORE DIAMETER (mm)		-90 80	DRILL LOGG				Peters O'She	
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Fracture Spacing Log (mm) 0 ²⁵⁰ 50	- o Non-intact Zone	Legend		Descriptio	on			Depth (m)	Elevation	Standpipe Details	SPT (N Value)
0							<u>×11/</u> ×	SYMMETRIX DRILLING	: No reco	overy, obse	rved by dri	ller	0.30	36.52		
	0.80							SYMMETRIX DRILLING	: Driller r	eports gree	nish pink ı	rock	0.80	36.02		
1		100	96	84		<u> </u>		Very strong, thickly to th pink/red/brown/grey/whi medium to coarse-grain slightly weathered.	e/orange	mottled, p	orphyritic, to locally				0 0	
	1.90							Dips are 35° to locally 8)°. Discol	ntinuities a	e widely to	b			0 0	
2		100	100	100		750	++ ++ +	medium spaced, rough Apertures are tight to pa	rtly open.	very thin b	anar. prown clay				0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
	2.90						} ' + ↓ + ↓ , +	smearing, slightly iron-o	kiue stain	eu.						
3		100	100	100			$^{+}_{++}$								0	
	3.70														• •	
4	4.30	100	100	93			} ' + + + + _ +								• 🗐 •	
	4.30	100	100	100											• •	
5						750	+'; +';									
	5.60	100	100	100									5 60	31.22	0 0 0	
	0.00							End of Borehole at	5.60 m				0.00			
6																
7																
8																
9																
													14/4			
	MAR e cas	-	.00-0).80r	n.					Sealed	Rise To	Time		mmen		JETAIL
									0.30	At N/S	0.30	<u>(min)</u> 5	S	eepag	e	
	T • • •								Hole	Casing	Depth to	0			VATER	DETAI
	TALI Date			epth	RZ Top RZ Bas	e	Ту	Date	Depth	Depth	Depth to Water	Com	iment	S		
	-02-1		5.0		1.00 5.60	-	50mn									



REPORT NUMBER

IGSL										10000	
CONTRACT	N6 Galway City Transport	Project - Phase 3						BOREHOL	E NO.	BH3/33	
CO-ORDINA	TES 532 101 71 E	RIG TYP	PE		Г	Dando 30	00 F	SHEET		Sheet 1 of 1	
	TES 532,101.71 E 728,307.46 N EVEL (m AOD) 35.60	BOREH	OLE DIAMI		1 m) 2	200 2.70		DATE COI DATE COI			
CLIENT	Galway County Council		MMER REF					BORED B	Y	WC	
INGINEER	ARUP	ENERG	Y RATIO (%	6)				PROCESS	SED BY	JL	
<u> </u>				_	(r			nples	-		e l
Uepth (m)	Description		pu	atior	u) u	ber	ble ble	٩	very	Field Test	Standpipe
Jepi			Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	Results	tano
	/ TOPSOIL			ш 35.45	 0.15	ш∠	50		Ŕ		S S
Firm lig	ht brown /grey slightly sandy g	ravelly SILT with	×0×××°	35.45	0.15	1					
a mediu	im cobble and boulder content					AA48863	В	0.50			
1	donas listé braum d'étais		ു×്×°	34.60	1.00	AA48864	в	1.00-1.45		N = 16	
· wealum	n dense light brown silty very sa	andy GRAVEL	0x 0x				_			(2, 3, 4, 4, 5, 3)	
			×0,0×0;0								
			X A X A o X o X			AA48865	в	2.00-2.45		N = 36	
2							5	2.50 2.40		(3, 3, 7, 8, 10, 11)	
				32.90	2.70						
	Borehole at 2.70 m										
3											
4											
5											
6											
7											
8											
-											
9											
-											
	ATA BORING/CHISELLING		<u> </u>						14/4	TER STRIKE DET	
	(m) Time Comments		Wate			Sealed	Ris			omments	
	2.7 1		Strike	e De	pth	At	To	(mi	<u>n) U</u>		
									1	lo water strike	
			1						GRO	UNDWATER PRO	GRE
NSTALLATI	ON DETAILS		Dat		Hole	Casing	De	pth to Co	omment		2.1
	Tip Depth RZ Top RZ Base	Туре			Depth	Depth		ater			
					0						
	Pit terminated at 2.70m on obs re-setup adjacent to hole.	truction. Borehole	е внз/33А		D - Small	le Legen Disturbed (tub)	d		UT - Uno Sample	listurbed 100mm Diameter	
					B - Bulk D LB - Large	e Bulk Disturber	d	+ Vial + Tub)	P - Undi W - Wat	sturbed Piston Sample	



REPORT NUMBER

	\checkmark	/															
	NTRA		Galway	City 7	Franspo	rt Project -							BORE		NO.	BH3/33A Sheet 1 of 1	
		NATES LEVEL (I	728	2,100. 8,308. 3	62 E 29 N 35.57			Pe Ole diam Ole dept			Dando 30 200 2.50	000	DATE (DATE (ED 04/02/2016 04/02/2016	
	ENT GINEEI		lway Co UP	ounty C	Council			MMER RE Y RATIO (%					BORED		BY	WC JL	
												Sa	mples			02	
Depth (m)			C	Descri	ption			Legend	Elevation	Depth (m)	Ref. Number	Sample Tvpe	-i		Recovery	Field Test Results	Standpipe Details
0		elly TOP						<u>x 1/</u> <u>x 1/</u> <u>y</u>	35.37	0.20	_						
	Firm a me	light brov dium cob	vn /grey ble and	slightl bould	y sandy er conte	r gravelly S ent	ILT with	× × × × × × × × × × × × × × × × × × ×	34 57	1.00							
1	Medi	um denso	e light br	rown s	ilty very	sandy GR	AVEL		-	1.00							
2									33.07	2.50							
	End	of Boreho	ole at 2.5	50 m													
3 4 5 6 7 8	End of Borehole at 2.50 m																
			Time					Wate	er Ca	asing	Sealed	Ris	se -	Time		TER STRIKE DET	AILS
	m (m) 2.4	To (m) 2.5	(h) 0.5	Con	nments			Strik		epth	At	T		(min)	+	omments	
																lo water strike	
										Hole	Casing		epth to	-		UNDWATER PRO	GRESS
				Tar	0	—		Dat	te	Depth	Depth		epth to Vater	Comr	ment	S	
	Date		pth RZ 1														
RE	MARK	S Boreho obstrue	ole BH3/3 ction. 2.0	33A so)hr mc	etup adj oving rig	acent to B out of field	H3/33 fol d with trac	lowing sha cked dum	allow per.	D - Sma B - Bulk LB - Lar	ple Legen all Disturbed (tub Disturbed ge Bulk Disturbe) ed		S P	ample - Undis	isturbed 100mm Diameter sturbed Piston Sample	
										Env - Er	nvironmental Sa	mple (Jar	+ Vial + Tub) V	V - Wate	er Sample	



REPORT NUMBER

0	අප	2											
:01	ITR/	АСТ	N	6 Ga	lway City Transpo	ort Pro	oject -	Phase 3	DRILLHOLE	NO		3/33R	
:0-	ORD	INAT	res		532,102.93 E							et 1 of	
				(mOE	728,306.15 N			RIG TYPE Knebel	DATE DRILL DATE LOGG			2/2016 2/2016	
				•	y County Council			FLUSHAir/MistINCLINATION (deg)-90	DRILLED BY			Peters	
	INE	ER		RUP	,			CORE DIAMETER (mm) 80	LOGGED B			O'She	
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Fracture Spacing Log (mm) 0 ²⁵⁰ 500	Non-intact Zone	Legend	Description		Depth (m)	Elevation	Standpipe Details	SPT (N Value)
0							<u>×17</u> ×	SYMMETRIX DRILLING: No recovery, obs	erved by driller	0.30	35.33		
1								as TOPSOIL. SYMMETRIX DRILLING: No recovery, obs as brown grey silty sandy gravel	erved by driller				
5								SYMMETRIX DRILLING: No recovery, obs as possible weathered rock recovered as b sandy gravel with cobbles	erved by driller prown grey silty	2.70	32.93		N = 50/2 mm (11, 14, 19, 5,
								SYMMETRIX DRILLING: No recovery, obs as rock	erved by driller	4.40	31.23		18, 5,
; {	5.10					<u> </u>	F	Medium strong to very strong, thick to thinl	v bedded	5.10	30.53		
;		100	97	88				blueish dark grey, fine grained, LIMESTON fossiliferous, localized chert and stylolites), locally moderately weathered. Dips are horizontal to locally vertical. Disco	NE (locally , slightly to				
	6.60							Dips are horizontal to locally vertical. Disco medium to closely spaced, rough to locally to occasionally undulose. Apertures are tig locally clay-filled.	smooth, planar				
	3.10	100	94	72									
3	3.10	100	100	95		540.0000							
ę	9.60												
	IARI						<u>] </u>			WA	TER ST	RIKE	DETAILS
lole	e cas	sed 0).0 <u>0-</u> {	5.10n	n.			Water Casing Sealed Strike Depth At	Rise Time To (min)		omment lo wate		e recorde
									Donth t-			VATER	DETAII
				ETAI				Date Hole Casing Depth Depth	Depth to Water Con	nment	S		
Ľ	Date	[ih Di	epin	RZ Top RZ Base	۳	Тур						



REPORT NUMBER

	පුප	Ŀ/														090	0
:01	NTR/	АСТ	N	6 Ga	Iway City Transp	ort Pro	oject -	Phase 3				DRII SHE	LHOLE	NO		3/33R et 2 of	
		DINAT		(mOI	532,102.93 E 728,306.15 N D) 35.63	3		RIG TYPE FLUSH			Knebel Air/Mist	DAT	e drille E logge		17/0	2/2016 2/2016 2/2016	;
		ER		ialwa RUP	y County Counci	I		INCLINATIO		m)	-90 80		LED BY			Peters O'She	
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Fracture Spacing Log (mm) 0 ²⁵⁰ 50	- o Non-intact Zone	Legend			Descrip	tion			Depth (m)	Elevation	Standpipe Details	SPT (N Value)
10	0.60	100	100	92				blueish da	ark grey, fin us, localize	ry strong, the grained, d chert and	LIMESTO	NE (locally					
11		100	98	94				Dips are h medium to	orizontal to closely sp nally undu	o locally ver baced, roug lose. Aperti	h to locall	y smooth, p	blanar				
2	1.90	100	100	88		580											
3 ¹	3.10	100	73	63				13.80-14.0	08m - Mod	erately wea	thered, sli	ght weake	ning.				
5	4.70	100	88	77		620.0000		-	04m - Clay	-filled fractu	ıre						
6	6.00	100	99	90		779.9999	99999999	- - - - - -									
17	7.00							End o	of Borehole	e at 17.00 n	1			17.00	18.63		
8																	
9																	
EN	/IARI	٢S			l		I	l						WA	FER ST	RIKE	DETAIL
lole	e cas	sed 0	0.00-5	5.10n	n.				Water Strike	Casing Depth	Sealed At	Rise To	Time (min)	-	mmen o wate		e record
														GRO	DUNDV	VATEF	
				ETA	ILS RZ Top RZ Bas		T\#		Date	Hole Depth	Casing Depth	Water		ment		tort last	dov drilling
	Date		ip Di	epin			Ту	19	18-02-16	11.90	10.60	5.10	vvater	ievel m	easured	start last	day drilling



REPORT NUMBER

/	\smile																
	NTRAC		6 Galw	vay City	Transpo	ort Project	t - Phase 3						BOREH SHEET		0.	BH3/42 Sheet 1 of 1	
	-ordin Ound	IATES LEVEL (534,72 726,82 D)	7.54 E 5.97 N 31.36			e Dle Diam Dle Dept		nm)	Dando 30 200 0.30	000	DATE C			22/03/2016 22/03/2016	
	IENT GINEEF		alway RUP	County	Council		SPT HAN	MER REI 7 RATIO (%	F. NO.				BORED PROCE		ЗY	WC JL	
_							1					Sar	nples				
Depth (m)				Desc	ription			Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recoverv		Field Test Results	Standpipe Details
0	TOPS	SOIL: So	oft dark	k brown	sandy g	ravelly Cl	LAY	VI IN VII	31.06	0.30							
2 3 4 5 6 7 8	Obstr End o	RD STRATA BORING/CHISELLING					ad										
			Tim	10				Wate	er Ca	asing	Sealed	Ris	se T	۱ Fime		R STRIKE DET	AILS
	. ,	To (m)	(h)	omments			Strik		epth	At	То		min)	Com	ments	
C).3	0.3	0.7	5												water strike	
		TIC:								Hole	Casing		epth to			IDWATER PRO	GRESS
		TION D			070	-		Dat	te	Depth	Depth		epth to Vater	Comm	ents		
	Date	∏ïp De	epth R	<u>к∠ Тор</u>	RZ Base	<u>e T</u>	уре	_									
RE	MARKS	3 1.5hrs	gettin	ng plant	and equ	ipment to	borehole l	ocation	I	B - Bulk LB - La	ple Legen all Disturbed (tub Disturbed rge Bulk Disturbe nvironmental Sa	ed	+ Vial + Tub)	Sar P -	nple	bed 100mm Diameter ed Piston Sample mple	
										1		(oai			2. 50		



REPORT NUMBER

	NTR/			6 Ga	lway City Transp 534,756.39 E	ort Pro	oject -	Phase 3				DRILLH SHEET			She	3/42R et 1 of 1	
GRO	OUN		VEL	(mOE	726,839.91 N) 32.65			RIG TYPE FLUSH			Knebel Air/Mist	DATE D DATE L	OGGE	ED	11/0	3/2016 3/2016	
	ENT Sinei	ER		alwa RUP	y County Council			INCLINATIO		m)	-90 80	DRILLE				. Peters . O'She	
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Fracture Spacing Log (mm)	Non-intact Zone	Legend			Descrip				Depth (m)	Elevation	Standpipe Details	SPT (N Value)
0								As TOPSC SYMMETF as brown o	DIL. RIX DRILL clay with co	ING: No rec obbles and	covery, obs boulders	erved by drill erved by drill	er		32.45		
2	2.30					<u> </u>		as possible SYMMETF as rock with SYMMETF	e weathere RIX DRILL th clay bar	ed rock ING: No rec	covery, obs	erved by drill erved by drill erved by drill	er	2.00	30.85 30.65 30.35		
3	3.90	100	89	38	Ę			grained, L and styloli Dips are 2 medium sp	IMESTON tes), fresh 0° to locall baced, rou	E (locally fo to slightly v ly 40°. Disc gh to locally	ossiliferous veathered. ontinuities y smooth, p	dark grey, fir localized cho are widely to blanar. brown clay	ert				
5	5.45	100	100	100	È	510		smearing.	are ugrit u	partiy ope	n, very unn	brown day				<u> </u>	
6		100	100	100	5											0 0 0 0	
7	6.90	100	100	100	Ŀ											0 0 0 0 0 0	
	8.45															0 0 0 0	
	9.85		100	100					f Doroh - 1	of 0.95					22.80	0 0 0 0	
	MARI e cas).00-2	2.30n	ı.			End o	Water Strike 2.30	e at 9.85 m Casing Depth 2.30	Sealed At N/S		Fime min) 20	Co	ommen Seepag		
	TALI Date			ETAI			Τ\/*		Date	Hole Depth	Casing Depth	Depth to Water	Com	ment			JETAI
	03-1		9.8		RZ Top RZ Bas 5.35 9.85		Typ 50mn										



REPORT NUMBER

1	\square															
со	NTRAC	T N6	Galway C	ity Transpor	t Project -							BOREH SHEET		Ю.	BH3/46 Sheet 1 of 1	
	-ordin Ound L	ATES _EVEL (n	728,3	751.37 E 890.26 N 29.88			e Dle Diam Dle Dept		mm)	Hand Du	-	DATE C			23/03/2016	
	ENT		-	ty Council								BORED			JD	
EN	GINEER	AR	JP			ENERG	(RATIO (%	//)				PROCE	SSED	BY	JL	
Depth (m)			De	scription			Legend	Elevation	Depth (m)	Ref. Number	Sample	Depth (m)			Field Test Results	Standpipe Details
0	conter limest	nt from 0 one.	.20m bgl.	nd a high co Cobbles and mestone roo	d boulders	boulder are of	<u>A IZ</u> <u>A IZ</u> <u>A IZ</u>	29.28	0.6	AA39980	В	0.00-0.6				
1 2 3 4 5 6 7 8	End o															
H	ARD STI	RATA BC	RING/CH	SELLING				L			1			WATE	R STRIKE DET/	AILS
Fro	m (m) ⁻	To (m)	Time (h)	Comments			Wate Strik	er Ca e D	asing epth	Sealed At	Ris To		Γime min)	Com	ments	
														No	water strike	
													G	ROUN	IDWATER PRO	GRESS
INS	TALLA	TION DE	TAILS				Dat	te	Hole Depth	Casing Depth	De W	epth to /ater	Comn	nents		
	Date	Tip Dep	oth RZ To	p RZ Base	Тур	De										
RE	MARKS	Hand d	ug pit at lo	ocation of BI	H3/46				B - Bu LB - L	nple Legen nall Disturbed (tub ilk Disturbed arge Bulk Disturbe Environmental Sa	ed	+ Vial + Tub)	Sa P	Imple	rbed 100mm Diameter ed Piston Sample ample	



REPORT NUMBER

1	বহ	2																	
0	NTR/	АСТ	N	6 Ga	lway City	Transpo	ort Pro	oject -	Phase 3						HOLE	NO		3/46R	
:0-	ORD	DINA	TES		531,749	9.54 E								SHEE		-0		et 1 of	
				(m. 0-	728,391	1.61 N			RIG TYPE			Casagra	inde		DRILLE			3/2016 3/2016	
		ULE		(mOE)) y County	29.81 Council				(doc)		Air/Mist			ED BY			SL	
	SINE	ER		alwa RUP	y County	Council			INCLINATION		m)	-90 80			ED BY			O'She	a
Ê	Ê													-					
pth (Core Run Depth (m)	%	%.	%.	Fract	ture	ЭС											tails	
De	Dep	T.C.R.%	S.C.R.%	.Q.D.%	Spac	cing	t Zol				Descrip	tion				~		e De	alue
Plot	Run	T	S	Ľ.	Lo (mr		ntac	pu								ш) с	ation	dpipe	> N
Downhole Depth (m)	Core				0 250	500	Non-intact Zone	Legend								Depth (m)	Elevation	Standpipe Details	SPT (N Value)
0	0				0 250	<u>ŭ</u>	~		SYMMETRIX	(DRILL	ING: No red	overv. ob	serve	d by dri	ller		ш	s k	0)
								0-0	as clayey gra	avel		-		•		0.50	29.31		
									SYMMETRIX as rock	(DRILL	ING: No red	covery, ob	serve	d by dri	ller			\mathbb{S}	
1	1.20															1.20	28.61		
									Very strong, grained, LIM	ESTON	E (locally fo	ssiliferous	s, loca	grey, f	ine hert				
		100	91	61			1		and stylolites	s), fresh	to slightly v	eathered.							
2		100	31						Dips are 20° medium spac	to locall	y 40°. Disc	ontinuities	are w	videly to r.					
	2.70							\square	Apertures are smearing.	e tight to	partly ope	n, very thi	n brov	vn clay					
								╞┯╧	sincariny.										
3		100	000					╞┯┿) ()	
		100	92	56				╞┯┿											
4	4.20																		
ľ	r.20																		
								╞╧┯										¥10,¥10,¥10,¥10,¥10,¥10,¥10,¥10,¥10,¥10,	
5		100	97	93				╞┼┰											
	5.70							┢┯											
6								╞┯╧										\mathbb{S}	
		100	99	93			k	╞┰┼											
							$\land \land \land$												
7	7.20																		
								╞╧┯											
8		100	100	92				╞┼┰											
	8.70																		
9																			
		100	100	100															
	/IAR			1 00-						Water	Casing	Sealed	Ris	20	Time				DETAIL
1016	e cas	seu U	.00-	1.20n	1.					Strike	Depth	At	T		(min)	Co	mmen	ts	
																N	o wate	r strike	record
	T	AT14	ם אר	ETAI	19					Date	Hole	Casing		epth to Vater	Com			VATER	DETAII
	Date				LS RZ Top	RZ Bas	e	Ту	be	Dale	Depth	Depth		Vater	Com	ment	5		
	03-1		16.0		11.50	16.40		50mn											



REPORT NUMBER

Je	35	5														000	
ON	TR/	АСТ	N	6 Ga	lway City Transp	ort Pro	oject -	Phase 3					LHOLE	NO		3/46R	
:0-0	ORD	INA	TES		531,749.54 E 728,391.61 N			RIG TYPE			Casagrar	do			14/0	et 2 of 3/2016	6
		D LE		(mOl	-			FLUSH			Air/Mist	DAI	E LOGGI			3/2016	5
		ER		ialwa RUP	y County Counci	I		INCLINATIO		m)	-90 80		LED BY			SL . O'She	a
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Fracture Spacing Log (mm) 0 ²⁵⁰ 500	Non-intact Zone	Legend			Descrip	tion			Depth (m)	Elevation	Standpipe Details	SPT (N Value)
10 1(0.20					-		Very stron	g, thick to IMESTON	thinly bedd E (locally fo	ed, blueish ssiliferous.	dark grey localized	, fine chert				
11	1.70	100	89	89		() / (and stylolit Dips are 2 medium sp Apertures smearing.	tes), fresh 0° to local baced, rou are tight to <i>(continue</i> d	to slightly v ly 40°. Disc gh to locall partly ope	veathered. ontinuities a y smooth, p n, very thin	are widely Ianar.	to				
12		100	95	95				- - - -								0 0 0 0	
13 1:	3.20					690											
	-															•	
14		100	65	41		719.9999	9999999	9								° = °	
	4.70																
15																0 0 0 0	
		100	100	100		k • • • •										o 🔤 o	
16 16	2 00																
16	5.20 6.40	100	100	100				Endo	of Borehold	e at 16.40 n	<u> </u>			16.40	13.41	1	
									Derener								
17																	
18																	
19																	
	ARI cas		.00-1	1.20r	n.				Water	Casing	Sealed	Rise	Time		TER S		DETAILS
-	_	-							Strike	Depth	At	То	(min)				e recorde
										Hole	Casing	Denth t	0 -			NATER	R DETAIL
	ate			epth	RZ Top RZ Bas	se	Ту	be	Date	Depth	Depth	Depth t Water	~ Com	ment	S		
15-0			16.0		11.50 16.40		50mr										



REPORT NUMBER

ONT	RACT	N6 (Galway (City Trans	port Projec	t - Phase 3	3					BOREHOL SHEET	E NO.	BH3/47 Sheet 1 of 1	
	RDINAT		728	058.02 E 289.22 N 37.02			pe ole diam ole dept		רm) מר	Dando 30 200 4.00	000	DATE COI DATE COI		ED 22/03/2016	
	IT NEER	Galv ARU	•	nty Counc	sil		MMER RE					BORED B		WC JL	
											San	nples			
			De	escription			Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	Field Test Results	Standpipe
) s	Soft dark	browr	n sandy g	gravelly C	LAY		<u> </u>	36.72	0.30				_		
F W	irm ligh vith a m	t browr edium	n slightly cobble a	sandy slig nd boulde	ghtly grave er content	IIY SILT	× × × × × × × × × ×		0.00	AA48890 AA48891	В	0.50		N = 8	
								2	2.00					(2, 2, 2, 2, 2, 2)	
? F g	Firm to s pravelly	tiff mot CLAY	ttled brov	wn slightly	sandy slig	Ihtly				AA48892	В	2.00-2.45		N = 12 (2, 3, 3, 2, 3, 4)	
3								5		AA48893	В	3.00-3.45		N = 20 (3, 3, 4, 5, 5, 6)	
	Dbstruct End of B		e at 4.00	m				33.02	4.00	-				N = 50/20 mm (25, 50)	
5															
5															
7															
3															
)															
					2								18/8		
Tom ((m)	Time	Commen			Wate			Sealed	Ris			TER STRIKE DET	AILS
3.9		4	<u>(h)</u> 0.75				Strik		epth	At	To	<u>(mi</u> ı	<u>n)</u>	lo water strike	
													GRO	UNDWATER PRO	GRE
						Type	Dat		Hole Depth	Casing Depth	De W	pth to ater Co	omment	ts	
Da				op RZ Ba	50	Гуре									
EMA	ARKS 1	.5hrs g	etting pl	ant and eo	quipment to	o borehole	location	I	D - Small B - Bulk I	Die Legen Disturbed (tub) Disturbed e Bulk Disturbe		I	Sample	listurbed 100mm Diameter sturbed Piston Sample	



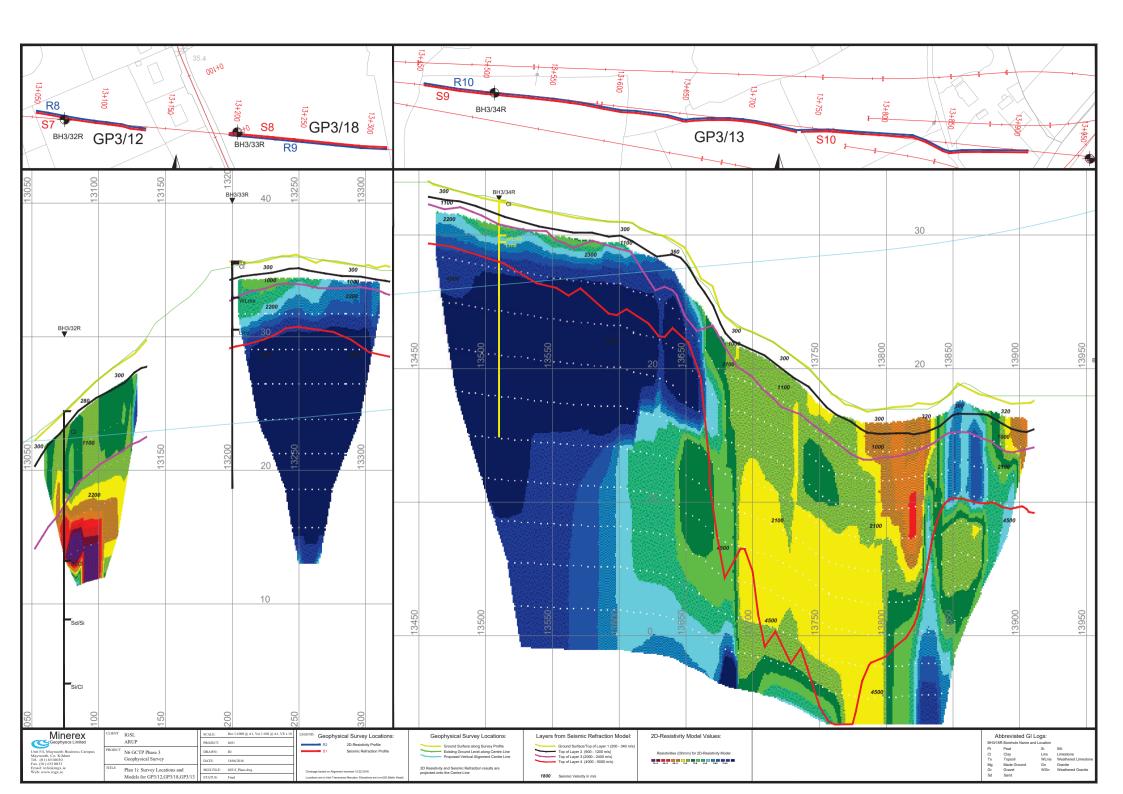
REPORT NUMBER

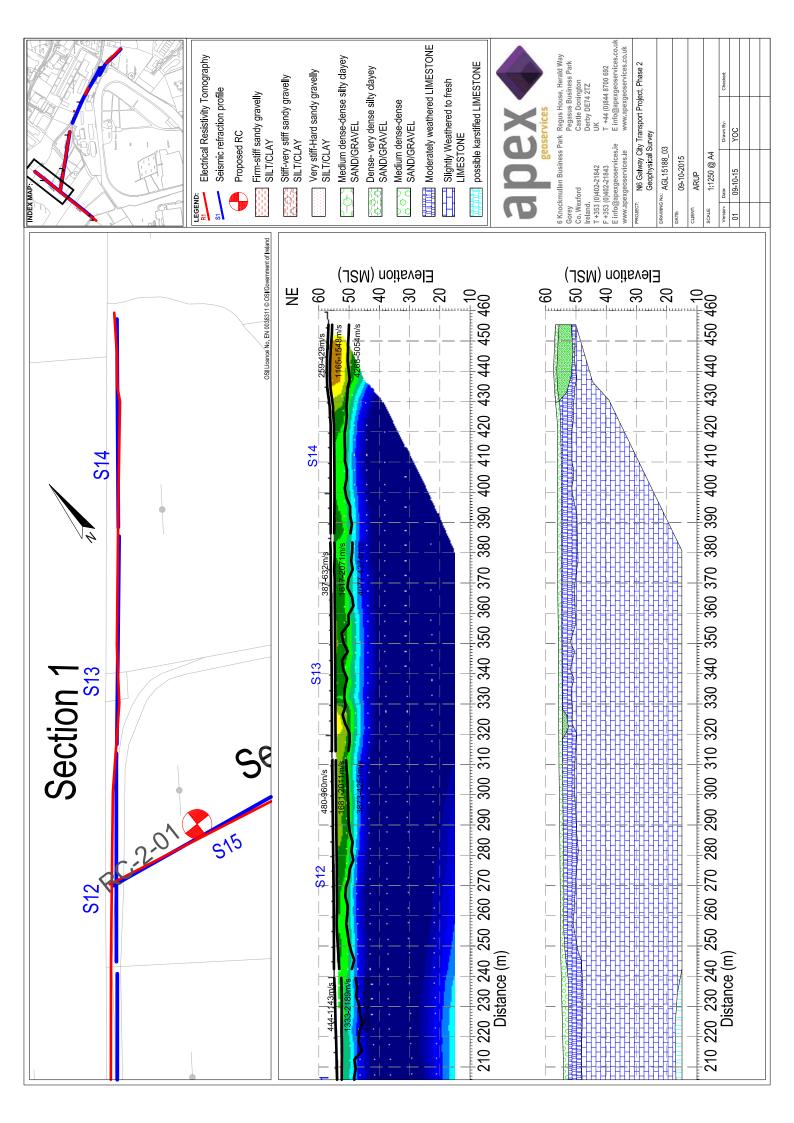
Je	33	<u></u> Ъ/															000	
CON	ITR/	АСТ	N	6 Ga	Iway City	Transpo	ort Pr	oject -	Phase 3					DRILLHOL	E NO		3/47R	
20-0	ORD		TES		533,062	37 F							F	SHEET			et 1 of	
				(728,286	.02 N			RIG TYPE			Casagra	unda	DATE DRIL)3/2016)3/2016	
GRC CLIE		JLE		(mOl	-	37.74 Council			FLUSH	ON (da)		Air/Mist	ŀ	DRILLED E			GSL	•
	INE	ER		aiwa RUP	y County (COUNCIL			INCLINATI		m)	-90 80		LOGGED E			-SL . O'She	ea
Ê	(L									, ,	-							
) Th (I	th (r	%	%	%	F arat		je										ails	
Downhole Depth (m)	Dep	.C.R.%	S.C.R.%	.Q.D.%	Fract Spac	ing	t Zor				Descrip	otion					Det	alue)
hole	Run	Ē.	Ś	2.	Log (mm		ntact	р р			200016				(m)	tion	lpipe	N Vŝ
Jown	Core Run Depth (m)				250	500	Non-intact Zone	Legend							Depth (m)	Elevation	Standpipe Details	SPT (N Value)
	0				0 250	500 1111111	Z	1	SVNNNET	י ווסח צוק	ING: No re	COVORY of	convor	by driller		ш	N N	S S
									as gravelly		ing. No re	covery, ob	sei veo	a by driller				
								F-	d									
1								Б.	4									
								0	d -									
2								0	4									
								0										
								P										
3								0										
								0	•									
								Þ	4									
• -									SVNANAET	י ווסח צוק	ING: No re	COVORY of	convor	by driller		33.54		
									as clayey	cobbles		-			4.50	33.24		
								<u>⊢</u> ⊥	SYMMETF as weathe		ING: No re	covery, ob	served	l by driller				
5																		
5	5.60							╞┯╧	Very strop	a thick to	thinly bedd	led blueid	h dark	arev fine	5.60	32.14		
3		100	87	07					grained. L	IMESTON	E (locally fo	ossiliferous	s. local	ized chert				
		100	0/	8/					1		to slightly							
6	6.60							╞┯┷	to medium	n spaced, r	ough to loo	ally smoot	th, plar	are widely nar.				
,		100	75	75					Apertures smearing.	are tight to	o partly ope	en, very thi	n brow	n clay				
							Kal										0 0	
	7.60							 	1									
3		100	95	75]									
2	3.60						l										0 0	
						-			-								0 0	
9		100	93	83				╞╧	-								ô <u>⊟</u> ô	
ç	9.60								1								o 🔤 o	
									1								• = •	
	IAR							·			0.		-			TER S	TRIKE	DETAILS
lole	cas	sed 0	.00-4	4.50r	n.					Water Strike	Casing Depth	Sealed At	Ris To			ommen	nts	
															1	lo wate	er strike	e recorde
												Cooir		nth t-			WATEF	R DETAII
				ETA		075	- 1			Date	Hole Depth	Casing Depth	De W	pth to /ater Co	mment	ts		
)ate 03-1		ip D 12.0		RZ Top 7.00	RZ Base 13.50	e	Ty 50mn										
		_		-														



REPORT NUMBER

10	ન્ટટા	5/															000	-
CON	ITR/	ACT	N	6 Ga	lway City	Transp	ort Pro	oject -	Phase 3				[RILLHOL	E NO	BH	3/47R	
20-0	ORD	INA	TES		533,062	2 37 F								HEET			et 2 of	
					728,286	5.02 N			RIG TYPE			Casagra	- do	DATE DRIL)3/2016)3/2016	
		JLE		(mOI	D) y County	37.74 Council			FLUSH INCLINATION	l (doc)		Air/Mist -90	L L	DRILLED I			SSL	•
	INE	ER		RUP	y County				CORE DIAME		m)	-90 80		OGGED			. O'She	ea
<u>و</u>	_ ۲																	
Downhole Depth (m)	Core Run Depth (m)	%.	%.	%.	Fract	ture	ре										tails	(
e De	Del	T.C.R.%	S.C.R.%	Q.D.%	Spac	cing	ct Zo				Descrip	tion				_	e De	/alue
loqu	Rur	Т	S	R	(mr		Non-intact Zone	pue							Depth (m)	Elevation	Standpipe Details	SPT (N Value)
Dov	Core				0 250	500	Non.	Legend							Dept	Elev	Stan	SPT
10		100	100	100	<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>		590		Very strong,	thick to	thinly bedd	ed, blueish	dark g	rey, fine				
1	0.60						-		grained, LIM and stylolites	i⊨STON s), fresh	⊢ (locally fo to slightly w	eathered.	, iocali	zed chert			• =•	
							770.0000	000000	Dips are 20°	to local	ly 40° & 80°	. Discontir	nuities	are widely	,		• = •	
11		100	100	100				╞┼┰	to medium s Apertures ar	e tight to	partly ope	ally smootl n, very thir	n, plan n browr	ar. ı clay			• =•	
1	1.60						549.9999	99999999	smearing. (c	continued	1)						• •	
12		100	100	100													° ⊒ °	
	2.60						660	╞┯┾									0 0 0	
																	0 000	
13		100	100	100													0 0 0	
1	3.50							╞╧╾	End of	Borehold	e at 13.50 n	า			13.50	24.24		
14																		
15																		
16																		
17																		
"																		
18																		
19																		
	IAR	(S													WA	TFR S		DETAILS
		-	.00-4	4.50n	n.					Water	Casing	Sealed	Rise		e co	ommer		
										Strike	Depth	At	To	(mir	1)			e recorde
																to wall	JUIN	
																יסואו		
NST	TALI		ON D	ETAI	LS					Date	Hole	Casing	Dep	oth to co	omment			
D	Date	T	īp De	epth	RZ Top			Ту			Depth	Depth	VV					
21-(03-1	6	12.0	00	7.00	13.50		50mn	n SP									

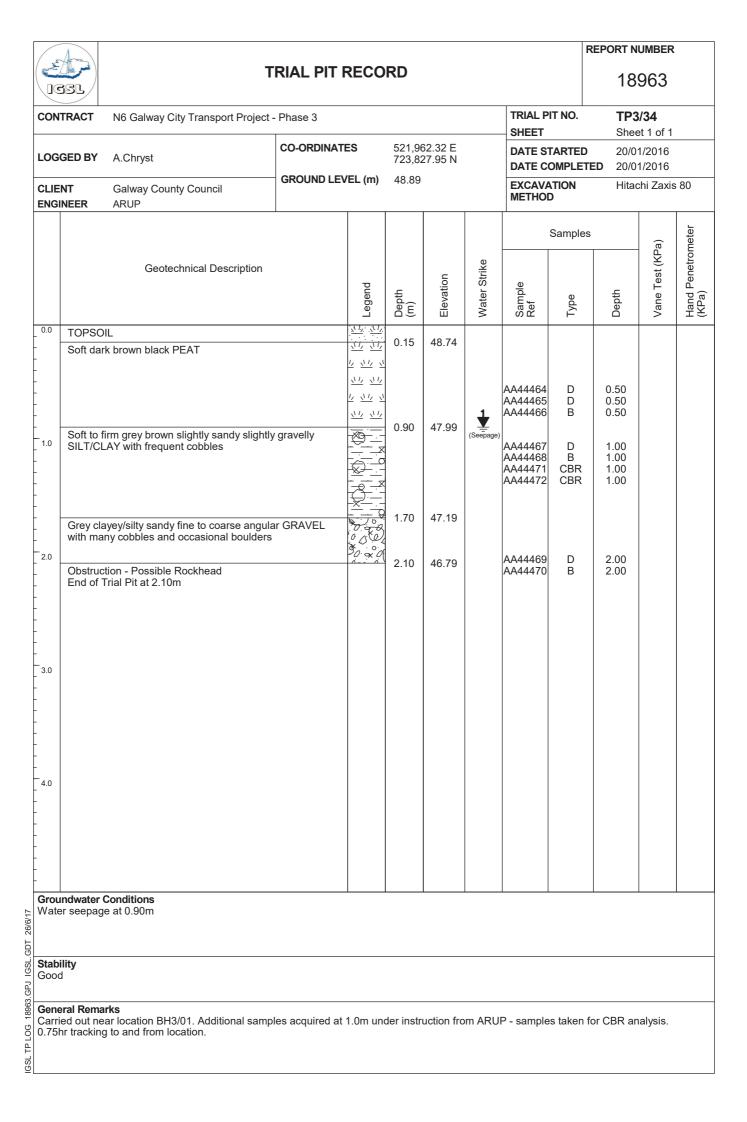




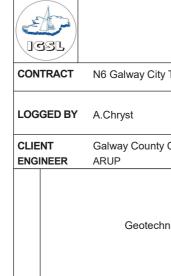
										REPORT NU	JMBER	
	JJ JSL	т	RIAL PIT F	RECO	RD					189	963	
CON	TRACT	N6 Galway City Transport Project	Phase 3					TRIAL P	IT NO.	TP3	/ 06 t 1 of 1	
LOG	GED BY	A. Chryst	CO-ORDINATI	ES	521,85 723,85	54.39 E 56.76 N		DATE ST		12/01	/2016 /2016	
CLIE ENGI	NT NEER	Galway County Council ARUP	GROUND LEV	'EL (m)	48.76			EXCAVA METHO	ATION D	Hitac	hi Zaxis	80
									Sample	s	a)	neter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Type	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
- 0.0	TOPSO			$\frac{\sqrt{I_z}}{\sqrt{I_z}} \frac{\sqrt{I_z}}{\sqrt{I_z}}$	0.20	48.56						
-	Son dari	k brown black fibrous PEAT						AA35350 AA37803	D B	0.50-0.50 0.50-1.10		
1.0 	Brown g angular	rey slightly clayey/silty sandy fine to GRAVEL with many angular cobbles	coarse		1.10	47.66	(Seepage)	AA37804 AA37805	D B	1.10-1.10 1.10-1.50		
-		ion - Possible Granite bedrock rial Pit at 1.50m			1.50	47.26						
2.0												
-												
- - - - - -												
-												
4.0												
-												
	ndwater (bage at 1.3	Conditions 00m										
Stabi Good												
Gene	eral Rema	ks										

IGSL TP LOG 18963.GPJ IGSL.GDT 26/6/17

	An									REPORT N	UMBER	
	3SL	1	RIAL PIT I	RECO	RD					18	963	
CON	TRACT	N6 Galway City Transport Project	- Phase 3					TRIAL P	PIT NO.	TP3		
LOG	GED BY	A. Chryst	CO-ORDINAT		521,7 ⁻ 723,62	18.09 E 27.37 N		- SHEET DATE S DATE C) 12/0 ⁻	et 1 of 1 1/2016 1/2016	
CLIE	NT	Galway County Council	GROUND LEV	/EL (m)	46.47			EXCAV		Hitad	hi Zaxis	80
ENG	NEER	ARUP						METHO	D			
									Sample	es	Pa)	ometer
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Type	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSO	IL		$\frac{\sqrt{1_{\ell}}}{\sqrt{1_{\ell}}} \frac{\sqrt{1_{\ell}}}{\sqrt{1_{\ell}}}$								
-	Grey bro	own silty very gravelly fine to coarse of granite.	SAND with	100 ×0	0.20	46.27		AA37806 AA37807	D B	0.20-0.50 0.20-0.50		
-		tion- Possible Granite bedrock			0.50	45.97		AA37007	В	0.20-0.30		
-	End of T	Frial Pit at 0.50m										
E												
1.0												
-												
-												
-												
2.0												
E												
-												
-												
3.0												
F												
F												
4.0												
-												
-												
-												
Grou	Indwater (Conditions										
Stab Good 0.50												
Stab Good												
Gen	eral Rema	rke										
0.50		rks nce required in tracking machine to	trial pit location.	0.25hr R	Reinstate	ement du	ring trac	k out of fie	eld			
i :												
2												



er er		г	RIAL PIT RE	CO	RD					REPORT N	umber 963	
	BGL		Dhara					TRIAL P		TP3		
CON	IRACI	N6 Galway City Transport Project						SHEET		Shee	et 1 of 1	
LOG	GED BY	A.Chryst	CO-ORDINATES		728,31	97.22 E 1.06 N		DATE S			1/2016 1/2016	
	NT INEER	Galway County Council ARUP	GROUND LEVEL	. (m)	56.03			EXCAVA METHO		Hitad	chi Zaxis	80
									Sample	s	_	eter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Type	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSO	L					8	х х Х	T	Ğ	>	ΪŤ
-	Soft bro	wn sandy slightly gravelly SILT	×o	× × × × × ×	0.15	55.88		AA33946 AA33947	D B	0.15-0.45 0.15-0.45		
-	Obstruc End of 1	tion - Possible Bedrock ⁻ rial Pit at 0.45m	¥	~~	0.45	55.58						
-												
1.0												
-												
2.0												
-												
-												
-												
3.0												
-												
-												
-												
4.0												
-												
-												
-												
Grou	Indwater (Conditions										
Stab	ility											
Gene	eral Rema	rks										
1hr t	racking in	total to and from trial pit location inc	clusive of reinstater	nent								



TRIAL PIT RECORD

	FRACT	N6 Galway City Transport Proj			F0.4			TRIAL PI			et 1 of 1	
.OGC	GED BY	A.Chryst	CO-ORDINAT		728,34	37.55 E 12.72 N		DATE ST DATE CO	ARTED		1/2016 1/2016	
CLIEN	NT NEER	Galway County Council ARUP	GROUND LE	VEL (m)	30.68			EXCAVA METHOD		Hitac	hi Zaxis	80
								5	Samples		a)	neter
		Geotechnical Descriptio	n	Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Type	Depth	Vane Test (KPa)	Hand Penetrometer
0.0	TOPSOI			<u>xn</u> <u>xn</u>	0.15	30.53						
	Firm dar rootlets	k brown slightly sandy gravelly S	SILT/CLAY with		0.10	00.00						
	Firm gre occasior	y brown slightly sandy slightly gr nal cobbles	avelly SILT with	×	0.60	30.08		AA44451 AA44452	D B	0.50 0.50		
1.0								AA44453 AA44454	D B	1.00 1.00		
2.0	Very stif gravelly	f grey and light brown slightly sa SILT with occasional cobbles ar	ndy slightly Id boulders		1.80	28.88		AA44455 AA44456	D B	2.00 2.00		
		tion - Possible Rockhead rial Pit at 2.40m		× , , , , , , , , , , , , , , , , , , ,	2.40	28.28						
3.0												
4.0												
Grou i Dry	ndwater (Conditions		<u> </u>								
Stabi Good	lity											
	ral Remai	rks J to and from location together w	ith padlock remove	al and reir	Istatem	ent upon (comple	tion				
		, <u></u> 2.12										

										REPORT N	UMBER	
	3SL	1	RIAL PIT	RECO	RD					18	963	
CON	ITRACT	N6 Galway City Transport Project	- Phase 3					TRIAL P	IT NO.	TP3		
LOG	GED BY	A.Chryst	CO-ORDINAT	ES	534,74 726,80	41.61 E 67.42 N		- SHEET DATE S DATE C) 22/03	et 1 of 1 3/2016 3/2016	
CLIE	INT	Galway County Council	GROUND LE	/EL (m)	31.74			EXCAV		Hitad	hi 13T	
ENG	INEER	ARUP						METHO	J			
									Sample	es	(Pa)	ometer
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Type	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSO Firm bro	L wn slightly sandy gravelly SILT with and boulders of limestone	many angular	× × × × × × × × × × × × × × × × × × ×	0.10	31.64		AA49484	В	0.10-0.50		
-	cobbles	and boulders of limestone			0.50	24.04						
-	Obstruct End of T	tion - Possible Rockhead Frial Pit at 0.50m			0.50	31.24						
-												
1.0												
-												
2.0												
-												
-												
_												
-												
-												
-												
4.0												
-												
-												
Ē												
	undwater (Conditions		<u> </u>	<u> </u>	1			<u> </u>		<u> </u>	
Dry Stab Goo Pit te												
ס ק Goo	ility d											
5 Gen	eral Rema	rks										
ן Pit te	erminated	on possible shallow rockhead										
- L												